

**DAMAGE ASSESSMENT OF OFFSHORE RISER-GUARDS
UNDER IMPACT LOADING**

By

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13865

Dissertation submitted in partial fulfillment of
the requirement for the
Bachelor of Engineering (Hons)
(Civil Engineering)

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CERTIFICATION OF APPROVAL

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Approved by,

(Dr. Zubair Imam Syed)

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TRONOH, PERAK

MAY 2014

CERTIFICATION OF ORIGINALITY

This is to certify that I am responsible to the work submitted in this project, that the original work is my own except as specified in the references and acknowledgements, and that the original work contained herein have not been undertaken or done by unspecified sources and person.

(THUANG CHEE KEE)

ABSTRACT

Steel riser-guards are installed on the offshore platforms to protect the risers against accidental vessel collisions. Offshore riser protection is important due to the flammable content it carries. In the absence of riser-guards, vessel collision can damage risers and hence leads to platform explosion. The conventional riser guards are designed to resist static force equivalent to vessel collision on any part of steel frame. Vessel collision with offshore structure has been highlighted in many design standards. However, those studies only limit to the structural response of the whole platform. In depth, investigation on the structural response and performance of riser guard has not been carried out. Therefore, this project was carried out to have better understanding on structural response of conventional riser guard in order to provide more economic and effective design of riser guard. In this project, finite element modelling of conventional riser guard under impact loading was performed using different Structural Analysis Computer System (SACS) modules to study its structural behavior. Different types of vessel collision scenario were studied in this research to understand the structural response and damage of steel riser-guards under different accidental vessel impacts. The riser-guards were modelled under impact load equivalent to vessel collision using SACS. The deformation, stress, strain and unity check values were analyzed to investigate the structural response and resulting damage to the riser-guards. Different magnitude of vessel impact loads were applied to the riser guard till its maximum capacity. The simulation shows that the maximum deformation occurs at the center of the riser guard. Collapse analysis was performed to study the plastic deformation of riser-guard and its plasticity. The structural response of offshore riser guard for different vessel collision scenarios was studied and presented in this paper.

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CHAPTER 1

INTRODUCTION

1.1 BACKGROUND OF THE STUDY

Offshore platforms receive frequent visits from vessel during its operation. Throughout its operation, offshore platforms always threatened by accidental load such as vessel collision. Uncontrolled accidental collision between the vessel and platform can results in fatalities and major economic loss. Various types of riser protection systems are introduced to prevent vessel colliding against offshore riser.

Riser-guards are tubular steel space frame installed on fixed jacket platforms to protect riser from accidental vessel collision. The conventional riser-guards are designed to resist static force equivalent to a collision [1]. In an event of collision, any plastic deformation of riser guard must be followed by maintenance work, which requires the removal and replacement of the steel tubular member [2]. Protection for the riser is extremely crucial as there are high volumes of extremely flammable fluids flowing through the risers. With the absence of riser-guards, accidental vessel collision against riser can cause oil spill and even result in explosion. The Mumbai High North Platform Disaster is the significant prove for the importance of riser guard in oil and gas industry [3]. As a precaution against vessel collision, the riser-guards are installed on offshore platforms on Malaysian water to provide protection against accidental impact load.

The conventional riser-guards shown in Figure 1 consist of welded steel tubular members forming a mesh-like structure, shielding the riser against vessel. The current design of conventional riser-guards is an adoption of boat fender design criteria. The design is plasticity based where it allows the riser-guard to undergo large deformation for higher energy dissipation and for reduction of impact force [1].



FIGURE 1: Conventional riser-guard

1.2. PROBLEM STATEMENT

The transport of hydrocarbons from subsea well to production or storage unit positioned at the sea surface is conducted by variety of risers [4]. The protection for risers is very vital to prevent casualties and economic loss due to accidental vessel collision against risers. The absence of riser-guard was made significant during the Mumbai High North Platform Disaster. The vessel collision resulted in 22 fatalities and a total damage amounting up to USD 195 million [3].

PETRONAS has been using a riser guard system that is similar to a boat fender on their fixed offshore platforms in Malaysian water. The conventional riser-guard adopts the design principle of boat fender as there is no riser-guard design standard established yet. The riser guard is designed to resist collision equivalent static force acting anywhere on the frame [1]. Vessel collision with offshore structures has been well studied and highlighted in many design manuals for offshore structures. However, there are no much studies on structural response and performance of conventional riser-guard. The adoption of the boat fender design criteria for conventional riser-guard can result in uneconomical design. Several issues have been raised over the use of conventional riser guard on fixed platforms [2]:

- i. *Excessive weight:* A 14m wide and 10m high conventional riser-guard system weighs up to 63 tonnes.
- ii. *Huge cost:* Due to its weight, floating vessel will have to be used during the installation of riser guard on platforms. The rate of floating vessel usage is estimated to be RM 500,000 (approximately 150,000 USD) per day.
- iii. *Time consuming:* The installation of conventional riser-guard requires welding work that is time consuming and also raises safety issues.

Therefore, damage assessment of offshore riser-guards under impact loading should be studied. Proper understandings of mechanism for transfer of reaction force to the jacket legs are very vital for further improvement of the riser-guard design.

1.3 OBJECTIVES

The objectives of the project are:

- To perform finite element modelling of conventional riser guards under impact loading.
- To determine the structural capacity of riser guard under accidental impact loading.
- To perform damage assessment of riser guard under vessel impact.

1.4 SCOPE OF THE STUDY

This study focused on the structural analysis of steel conventional riser guard under accidental vessel impact load. The scope of analysis as of this report covers from the Linear Static in Place analysis, Ship Impact Analysis and the Non Linear Collapse analysis. This research will only focus on the assessment of conventional steel riser guard where any new innovation of riser guard is considered beyond the scope of study. The conventional riser guard will be simulated under impact loading equivalent to a vessel collision using Structural Analysis Computer System (SACS) modules. In this study, other loadings such as wave, current and sea-state load are considered negligible as compared to accidental vessel impact load. Thus, the effect of these loadings on performance of offshore riser guard is considered out of the scope. Non-linear structural analysis of riser guard will be performed in the study while experiment variation of the finite element result is considered beyond the scope of study. The structural behavior of conventional riser guard is studied and analyzed in accordance to American Petroleum Institute (API) standard and PETRONAS Technical Standard (PTS). The deformation, and stress of the model are analyzed to study the plasticity characteristic of riser guard and unity check will be performed to determine the actual capacity of the riser guard. The global deformation of offshore riser guard under impact loading was studied but localized deformation due to local denting was beyond the scope of the research.

CHAPTER 2

LITERATURE REVIEW

Continually growing world energy markets lead to increase of demand on oil and gas production. Offshore platforms contribute up to 30% of the world's oil production [5] and is needed for oil and gas exploration and production. Today, there are over 11095 offshore platforms around the world in water depth up to 2280m [5]. The riser protection system was introduced to protect the riser against vessel collision. This chapter summarizes various aspects of ship-platform collision, collision mechanics and review past research in the related area.

2.1 TYPES OF OFFSHORE PLATFORM

There are different types of offshore platforms depending on the depth of the water. The offshore platform can be classified into fixed structures and floating structures. Tables 1 summarize the total number of operational platform in world in year 2012. Researches have shown that 96.4% of offshore platforms are fixed platform [5].

TABLE 1: Operational Platform in year 2012 [5]

Type of Platform	Number	Percentage
Fixed Platform	10700	96.4%
Floating Platform	395	3.6%
Total	11095	100.0%

Fixed structures are extending to the seabed such as Fixed Jacket Platform, Compliant Tower, Gravity Based Structure (GBS) and Jack-up barges. Furthermore, floating structure is structure that float near the water surface [6]. Examples of floating structure are semisubmersible, spar, tension leg platform (TLP), floating production system (FPS) and FPSO [6]. Figure below shows different types of offshore platform in different water depth.

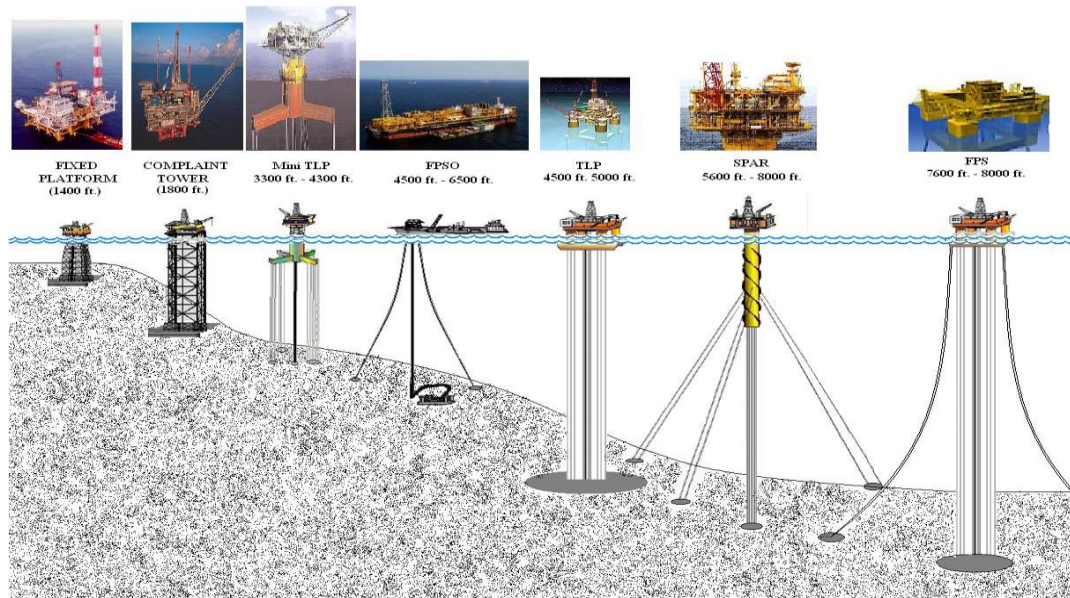


FIGURE 2: Different types of offshore platform

2.2 SHIP-PLATFORM COLLISION

Offshore platform will receive frequent visit from different type of vessels and vessel collision against offshore platform is unavoidable. Based on the database of collision incident produced by Offshore Division of the Health and Safety Executive, the summary of reported collision incident in UK Continental Shelf is shown in Table 2 [7].

TABLE 2: Reported Collision Incidents in the UK Continental Shelf from Year of 1975 to 2001 [7].

Vessel Type	Supply Vessel	Stand-by vessel	Attendant vessels	Passing vessels	Unspecified vessels
Number of Incidents	353	87	74	8	35
Percentage of Occurrence	63.4%	15.6%	13.3%	1.4%	6.3%

Table 2 indicate that there are total number of 557 incidents involving vessels and fixed platform from year 1975 to 2001. A review conducted based on the collision incident and the damage resulting from the collision incident from year 1975 to 2001 is summarized in Table 3 [7]. From the database, 17 incidents were classified as severe damage, 69 as moderate and 322 as minor. This indicates that 83.3% of

collision incident will cause damage[7]. Therefore, the vessel collision against riser should be minimized by introducing several types of protection system.

TABLE 3: Damage resulting from Incidents in UK Continental Shelf from Year 1975 to 2001 [7]

	Damage Class				
	None	Minor	Moderate	Severe	Unspecified
Number	93	322	69	17	56
Percentage	16.7%	57.8%	12.4%	3.1%	10.1%

The extent of damage class and the criticality of the member involved is described as below [7]:

- i. Severe: Damage affecting the integrity of an installation sufficient to require repair in the immediate or short term (up to 1 month). Where the actual date of repair could not be determined then the criticality of the damage damaged member was considered where this was available. In the absence of other repair information damage to non-redundant members was considered severe;
- ii. Moderate: Damage requiring repair in the medium (up to 6 months) or longer term (over 6 months);
- iii. Minor: Damage not affecting the integrity of the installation;
- iv. None: No damage occurred;
- v. Unspecified: Damage believed to have occurred but was not specified in reports

2.3 COLLISION MECHANICS

One of the main approaches for estimation of vessel collision is the Impulse-Momentum approach [8]. This approach equates the impulse force to the change of in momentum of the impacting vessel as shown in the following equation [9]:

$$\text{Impulse force, } F = \frac{m\alpha(v_i - v_f)}{t}$$

Where,

m = mass of vessel (kg)

α = added mass coefficient (1.4 for broadside impact and 1.1 for bow/stern impact)

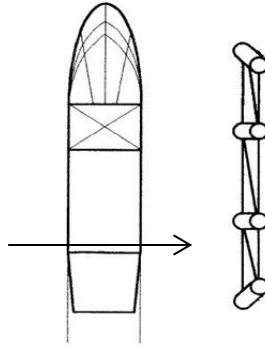
v_i = initial vessel velocity (m/s)

v_f = final vessel velocity (m/s)

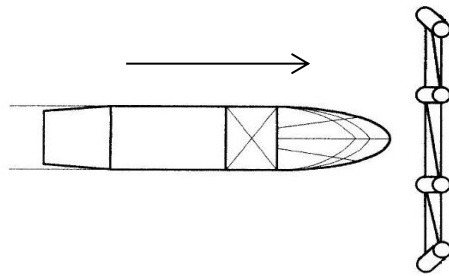
t = time taken for the vessel to stop (s)

In reality, three possible vessel collision scenarios that may occur are [10]:

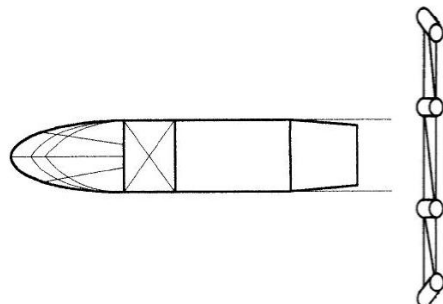
- i. Broadside impact



- ii. Bow impact



- iii. Stern impact



Bow collision is more significant as compare to others collision as it induces larger vessel impulse force and result in more severe damage to the riser guard.

During vessel collision, an offshore riser guard absorbed the impact energy from ship by undergoes [11]:

- i. local denting
- ii. elastic beam bending
- iii. Global structural deformation (elastic and plastic).

2.4 RISER SYSTEMS

A riser system is essentially conductor pipes connecting floaters on the surface and the wellheads at the seabed [12]. Riser system plays an important role in ensuring safety in all phases from drilling, completion, production to export [12]. Offshore riser is used to transport oils or gas from subsea oil well to platform. Additional functions of riser are provided as follows [13]:

- Conveys fluid between the wells and the floater for production and injection risers.
- Export fluid from floater to pipeline for export riser.
- Guide drilling or work over tools and tubular to and into the wells for drilling and work over riser.

2.5 RISER PROTECTION SYSTEM

Protection for risers is vital as to prevent vessel collision against riser. The vessel collision will result in huge explosion that cause fatalities due to the flammable contents it transport. Mumbai High North Platform disaster in July 2005 has raised the awareness on importance of riser protection system. With the absence of riser protection, this vessel collision accident in Mumbai High North Platform hits the riser and causing massive fire which destroyed the platform within 2 hours [14]. This accident cause major disruption and 22 fatalities and total damage up to USD 195 million[3].

As a lesson learnt from Mumbai High North Platform disaster, most of the offshore platforms are installed with riser protection system. Among the riser protection used are conventional riser guard system on fixed oil platform [1]. Riser protection net (RPN) [15], Marine Riser Protector [16] and Geobrug's GBE system [17].

TABLE 4: Comparisons for different type of riser protection

Type of Riser Protection	Advantages	Disadvantages
Riser Guard	Can undergo large deformation for high energy dissipation [1]	Excessive weight, huge cost, time consuming and raises safety issues [1]
Riser Protection Net (RPN)	Capable of bearing maximum tensile load up to 650 tonnes [15].	Undergoes large deflection and is not apposite for fixed platform [15].
Marine Riser Protector	Can divert ice floes away from riser. [16]	Receive high wave impact and easily to detached [16]
Geobrug's GBE system	Lighter and easier to attach Can sustain impact energy up to 8000 KJ [17]	Can only be used if the deflection upon impact can be controlled [17]

Table 4 compares the advantages and disadvantages of different type of riser protection system. However, the actual performance of these riser protection systems is not well studied. Better understanding of structural behaviour of riser protection system can provide more economical design of riser protection system.

CHAPTER 3

METHODOLOGY

This chapter presents the methodology that was followed in this study to attain the well-defined objectives for this research.

3.1 RESEARCH METHODOLOGY AND PROJECT ACTIVITIES

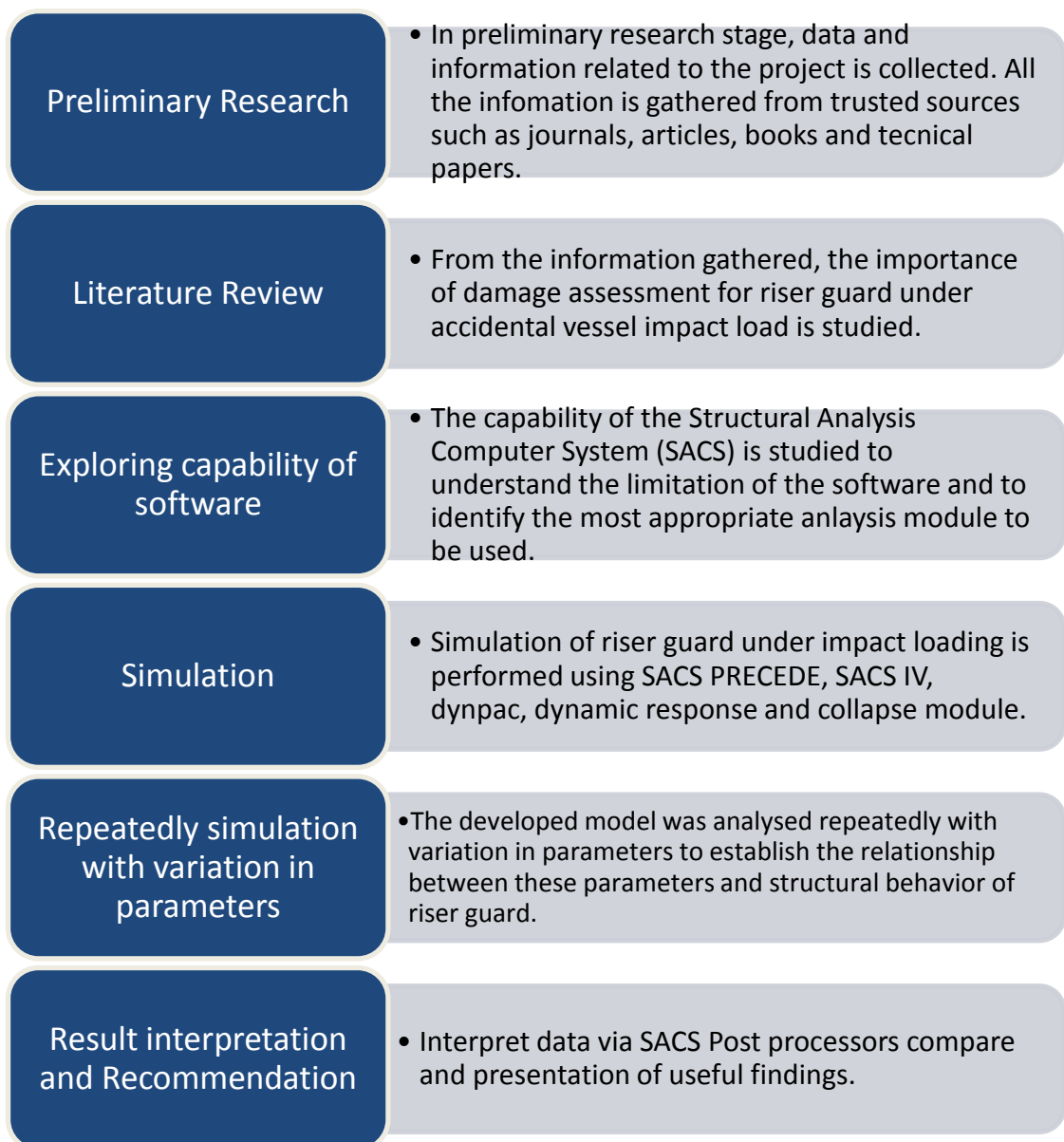


FIGURE 3: Project Activities

3.2. PARAMETRIC STUDY

A detail systematic parametric study was conducted to study various aspects associated with structural behaviour of offshore riser guard under vessel impact. The model was developed using SACS 5.3 software to stimulate the effect of accidental vessel impact on riser guard. Several important parameters were identified to provide better understanding of accidental impact phenomena as listed in Table 5. The developed model was analysed repeatedly with variation in parameter mentioned to study the sensitivity of these parameters. The results obtained from the parametric study were used to achieve the objectives of this project.

TABLE 5: Parametric Study

No	Parameters
1	Vessel mass
2	Type of collision
3	Velocity of vessel

3.3 MODELLING AND SIMULATION APPROACH

For the purpose of this project, SACS 5.3 Suite of Programs will be used extensively for both modelling and simulation. Several SACS modules will be used herein. The first is the PRECEDE program, to be used as the graphical user modeller. DYNPAC will be employed to generate the dypnac mode shape and mass file which is required for ship impact and collapse analysis. The DYNAMIC RESPONSE module will be used to simulate ship impact analysis while the COLLAPSE module will be used to perform the non-linear collapse analysis. The results can then be viewed in SACS post processors such as POSTVUE and COLLVUE which enables the author to interpret the results interactively and graphically. The general flow of the SACS modelling and simulation is illustrated as diagram below:

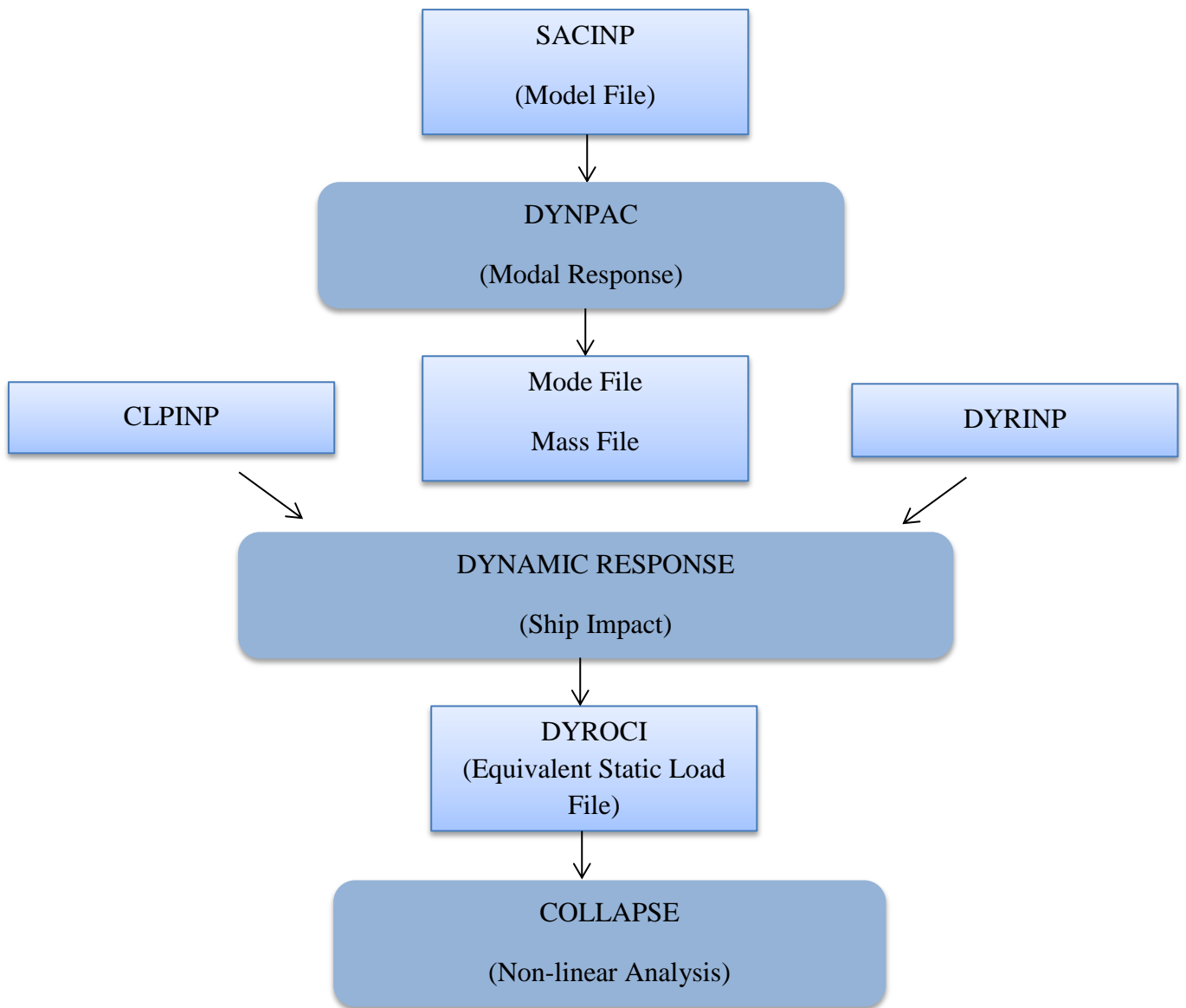


FIGURE 4: Flow Chart of SACS Simulation

3.3.1 SACS Modelling

The 14m wide and 10m height conventional riser guard is modeled in PRECEDE based on typical riser guard design to study its performance during accidental impact loading. The following shows the isometric view, front view, back view, side view and plan view of the typical conventional riser guard in SACS.

3.3.1.1 Isometric View

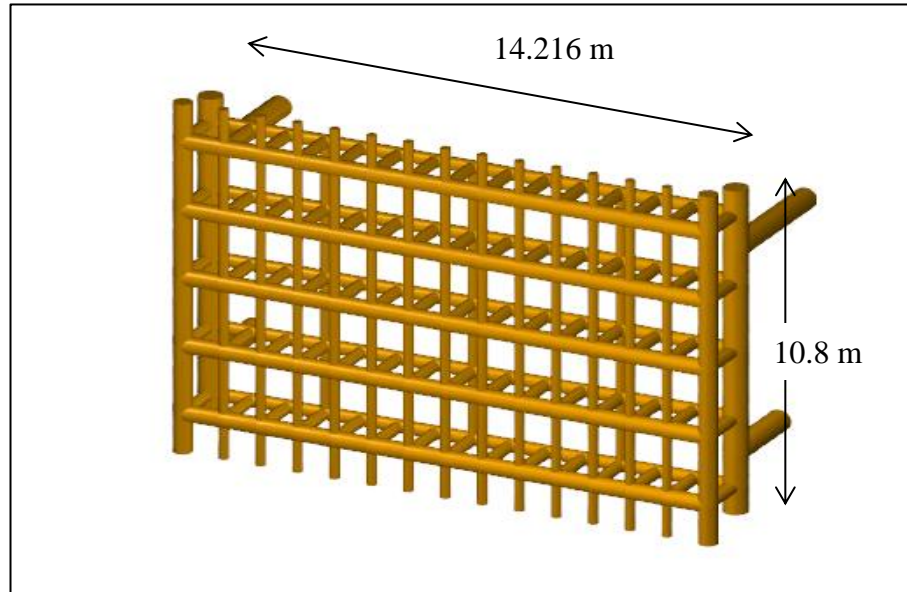


FIGURE 5: Isometric View of Riser Guard

3.3.1.2 Front View

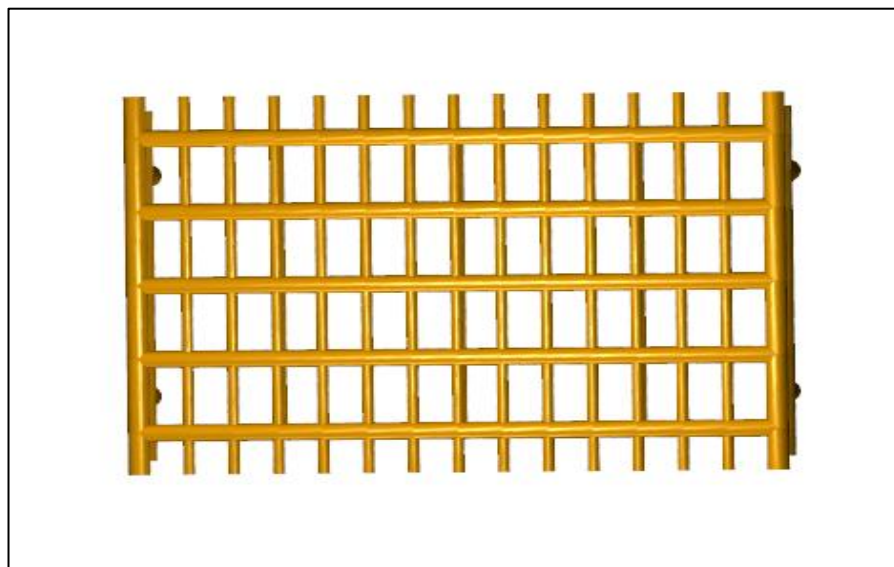


FIGURE 6: Font Elevation of Riser Guard

3.3.1.3 Back View

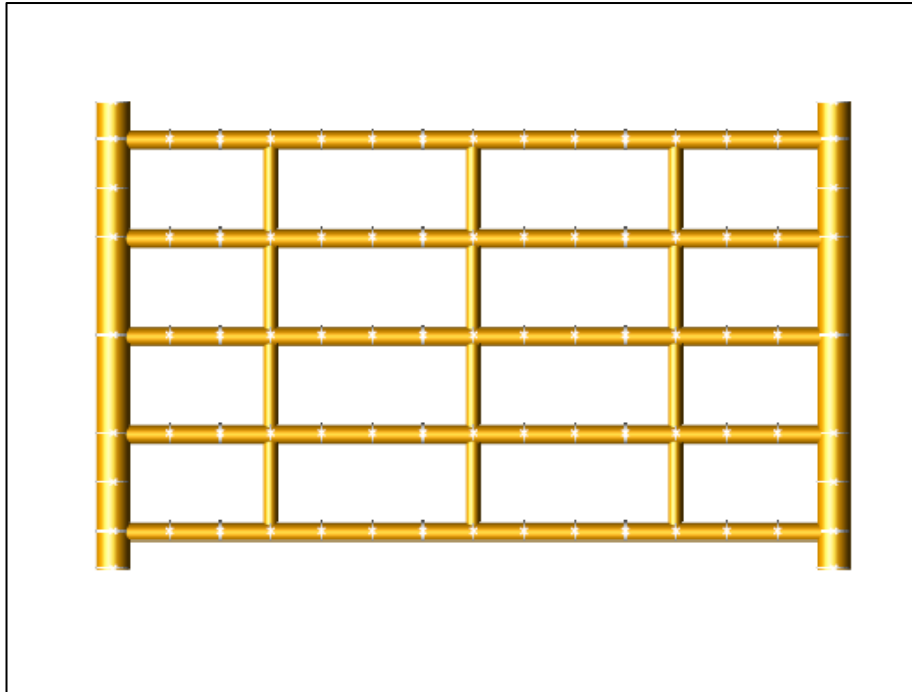


FIGURE 7: Back Elevation of Riser Guard

3.3.1.4 Side View



FIGURE 8: Side Elevation of Riser Guard

3.3.1.5 Plan View

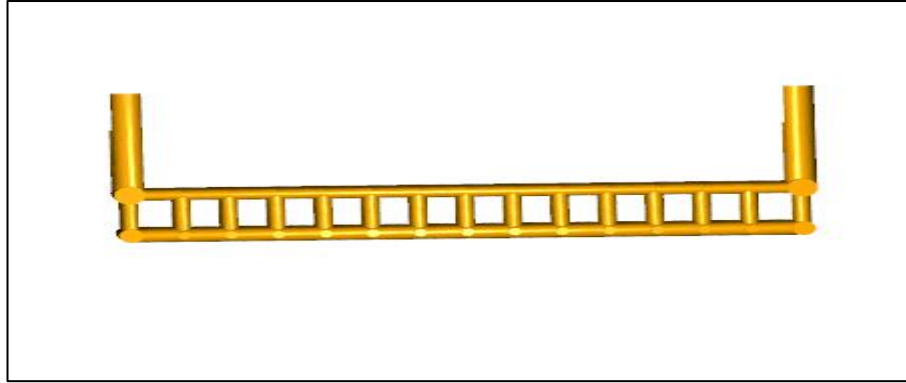


FIGURE 9: Plan View of Riser Guard

3.3.1.6 Fixity and Master Degree of Freedom (MDOF)

The support ends are modelled as completely fixed in all translational and rotational as specified as “111111”. Master degree of freedom (MDOF) is specified at joint 0184, 0185, 0186 and 0187 as shown in Figure 10 where the joint Degree of freedom (DOF) is retained in all direction of translation (“222000”). Point mass related to translational DOF while inertia related to rotational DOF. In my study, rotary inertia effect has less significant as compared to mass effect. Hence, DOF is only retained in translation. The other nodes will act as slave degree of freedom. MDOF will control the vibration and the remaining DOF will just follow the pattern defined by the response of master node.

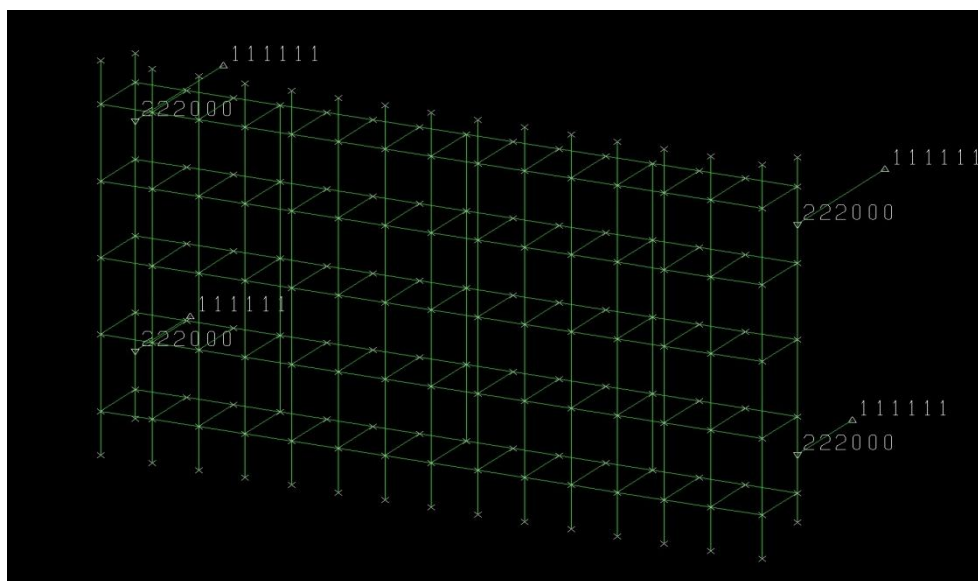


FIGURE 10: Joint Fixity and Master Degree of Freedom

3.3.2 DYNPAC Extract Mode Shape Module

Dynpac Extract Mode Shape Module is performed to generate the mode shape and mass file which is required for collapse analysis. SACINP is required in performing this simulation.

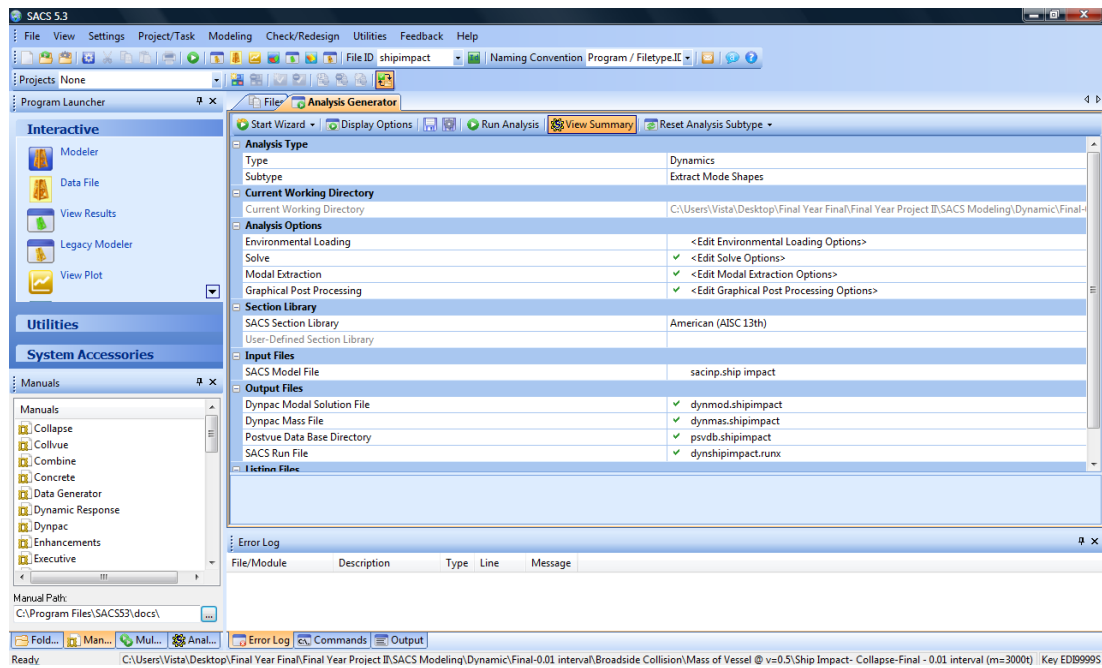


FIGURE 11: Dynpac Extract Mode Shape Module in SACS

3.3.3 Dynamic Response (ship impact analysis)

Once the dynpac mode shape and mass file is generated, the dynamic response (ship impact) file DYRINP is created as below:

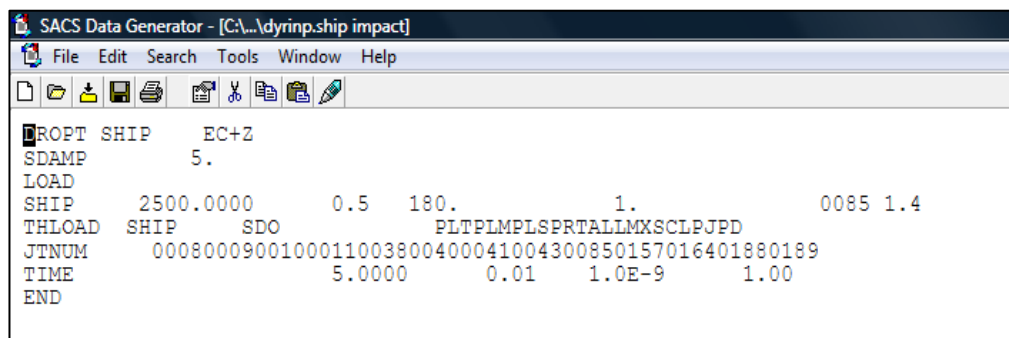


FIGURE 12: Dynamic response (ship impact) file

This analysis will generate DYROIC (equivalent static load) file where the inertial load correspondent with specific vessel mass and velocity is generated.

3.3.4 Collapse Analysis using Dynamic Response Inertial Loading

Collapse input file (CLPINP) have to be created prior to any collapse analysis. The CLPINP file created for the simulation of damage assessment or riser guard is shown in figure below:

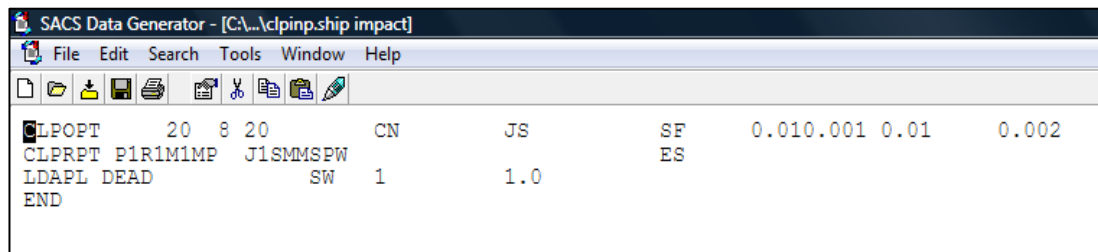


FIGURE 13: Collapse input file

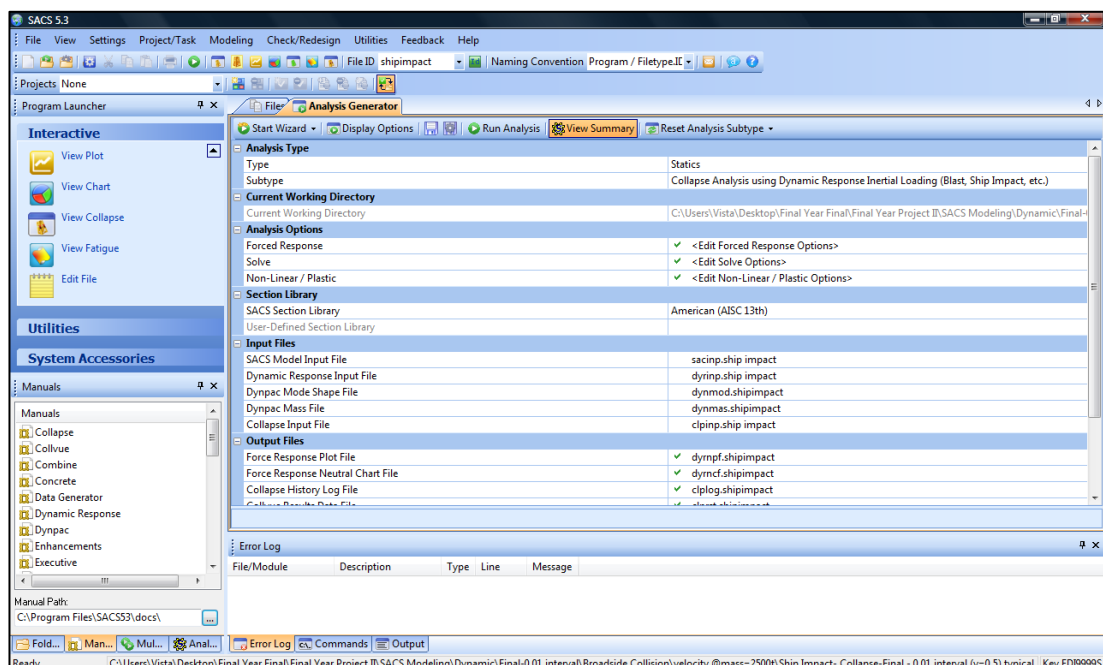


FIGURE 14: Collapse Analysis using Dynamic Response Inertial Loading

3.4 KEY MILESTONES & GANTT CHART

Week No	FYP 2													
Progress	1	2	3	4	5	6	7	8	9	10	11	12	13	14
Non-linear Structural Analysis of Riser Guard	●	●												
Selection of suitable Master Degree of Freedom for the riser guard					●									
Ship impact analysis of riser guard under different collision scenario					●	●								
Submission of Progress Report							●							
Result and Data Analysis							●							
Determination of Actual Capacity of Riser Guard								●						
Pre-SEDEX														
Submission of Draft Final Report														
Submission of Dissertation (Soft Bound)														
Submission of Technical Paper														
Project Viva														
Submission of Dissertation (Hard Bound)														
<p>● : Understanding of non-linear structural analysis</p> <p>● : Study the performance of riser guard</p> <p>● : Study the structural behavior of riser guard under different magnitude of impact load</p>														

CHAPTER 4

RESULTS AND DISCUSSION

4.1 INTRODUCTION

SACS 5.3 Suite of Programs will be used extensively for both modelling and simulation. Several SACS modules were used to perform damage assessment of riser guard under impact loading. SACS IV module was used to perform static analysis in order to determine the structural capacity of riser guard under vessel impact. DYNAMID RESPONSE module was employed to perform ship impact analysis while COLLAPSE modules for collapse analysis. Ship impact analysis was conducted to study the ship impact scenario as well as generate equivalent static load for collapse analysis. Collapse analysis was employed to study the linear and non-linear behaviour of riser guard. The results obtained from these simulations were interpreted and presented in this chapter.

4.2. STATIC IN-PLACE ANALYSIS

Static analysis of offshore riser guard under loading equivalent to vessel collision is performed using SACS. The static analysis was performed repeatedly with increment in load to determine the structural capacity of offshore riser guard under accidental vessel impact load. Different magnitudes of vessel impact load are applied to the riser guard to study its structural response.

4.2.1 Vessel Impact Load

The impact load equivalent to vessel collision is applied to the riser guard. In actual case, vessel impact will only hit part of the riser guard. Therefore, in this study, vessel impact load is assumed to be only applied at the center of the riser guard as illustrated in Figure 11.

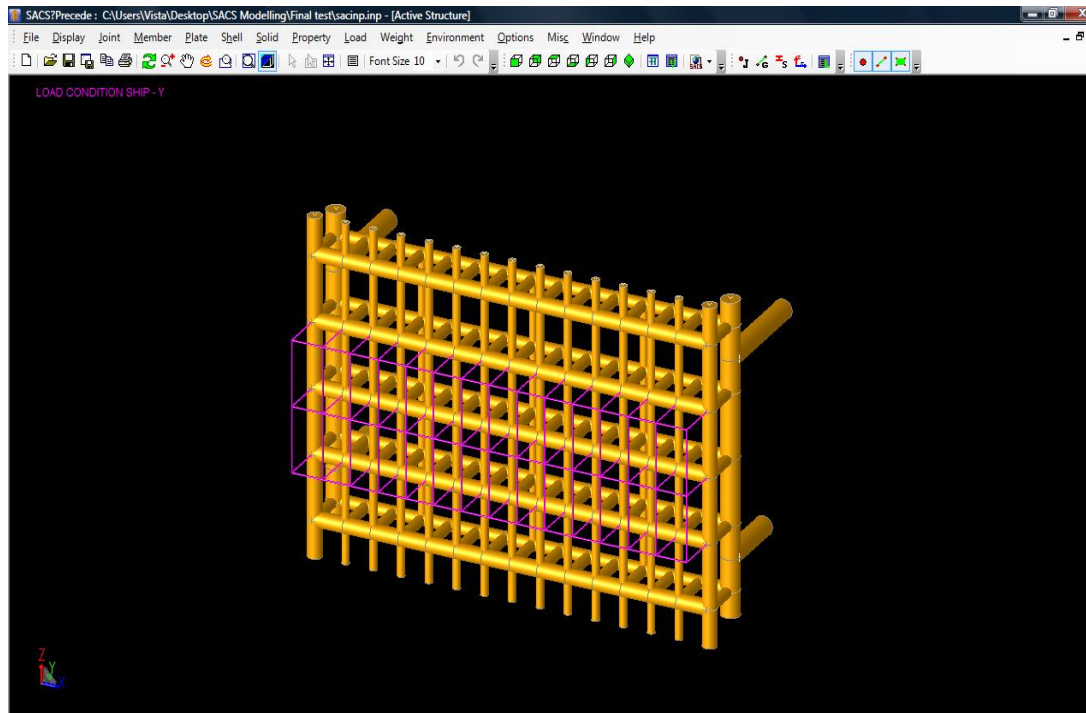


FIGURE 15: Vessel Impact Load on Riser Guard

4.2.2. Unity Check

Member Unity Check

The figure shows the member unity check for riser guard when total of 6560 KN area load is applied at the center of the riser guard. Member with red color indicates that the member is structurally failed as the unity check is more than 1. Figure 12 shows that all the members have passed the member unity check.

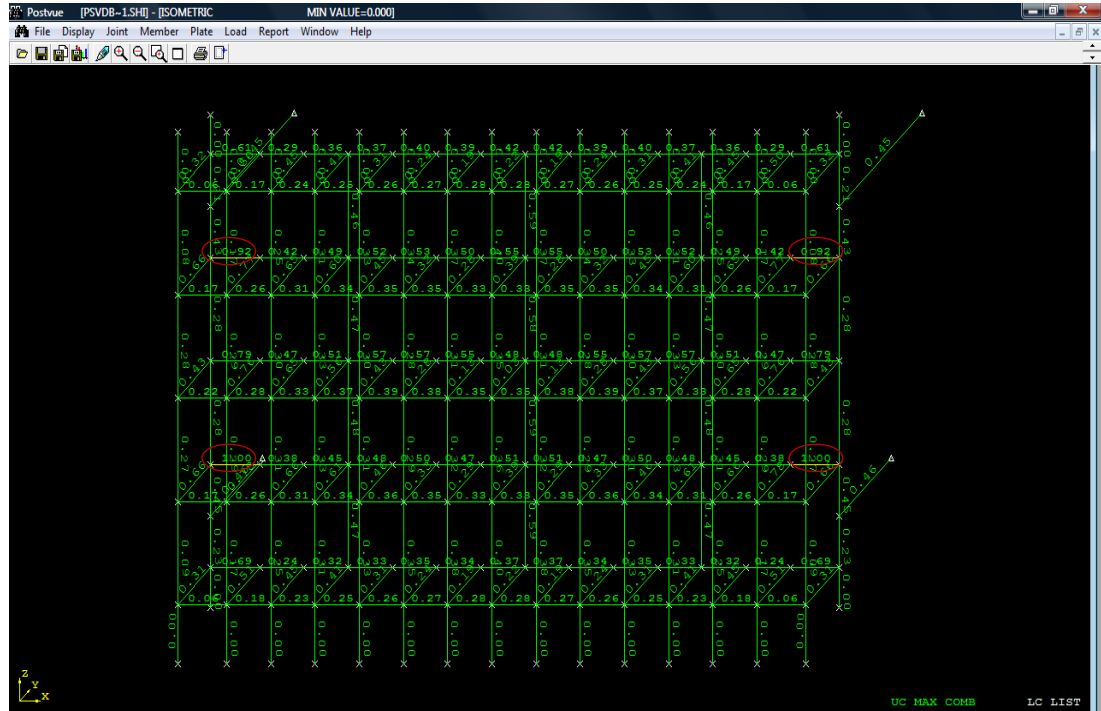


FIGURE 16: Member Unity Check for Riser Guard with applied load of 6560 KN

Member 0165-0166, member 0178-0179, member 0135-0136 and member 0148-0149 have higher unity check as compared to other tubular member. The unity check for these members is summarized as shown in table below:

TABLE 6: Member Stress at Critical Member

Member	Load Condition	Axial Stress (N/mm ²)	Bending Stress	
			Y (N/mm ²)	Z (N/mm ²)
0135-0136	C1	-3.91	48.22	226.08
0148-0149	C1	-3.91	48.22	226.08
0165-0166	C1	-12.7	-57.07	233.73
0178-0179	C1	-12.7	-57.07	233.73

TABLE 7: Member Unity Check at Critical Member

Member	Critical Condition	Load Condition	Unity Check			
			Axial	Bend-Y	Bend-Z	Total
0165-0166	C<.15	C1	0.063	0.053	0.885	1.00
0178-0179	C<.15	C1	0.063	0.053	0.885	1.00
0135-0136	C<.15	C1	0.019	0.039	0.861	0.92
0148-0149	C<.15	C1	0.019	0.039	0.861	0.92

From Table 7, member 0165-0166 and member 0178-0179 have the highest unity check. In other word, failure will be occurred at member 0165-0166 and member 0178-0179 prior to other tubular members under combine loading. Compression with axial load ratio <0.15 is critical for the failure.

Joint Unity Check

Welding point is the weakest part of the offshore structure. Hence, joint unity check is crucial in offshore structure analysis. Joint can program is developed in SACS to perform the joint unity check in order to determines the adequacy of simple and overlapping tubular joints for punching shear. Area load of 6560 KN is applied to the centre of riser guard and the joint unity check is performed. Joint failure occurred if unity check is more than 1. Table 7 summarize the joint unity check at critical joint if 6560 KN area load is applied.

TABLE 8: Joint Unity Check at Critical Joint

Joint	Diameter	Thickness	Yield Stress	UC
136	40.64	1.57	345	0.96
148	40.64	1.57	345	0.96
68	50.8	1.905	345	0.935
104	50.8	1.905	345	0.935
157	40.64	1.87	345	0.917
66	50.8	1.905	345	0.901
102	50.8	1.905	345	0.901
168	40.64	1.57	345	0.89
176	40.64	1.57	345	0.89
138	40.64	1.57	345	0.878
146	40.64	1.57	345	0.878
167	40.64	1.27	345	0.849
177	40.64	1.27	345	0.849
108	40.64	1.57	345	0.84
116	40.64	1.57	345	0.84
142	40.64	1.87	345	0.838
172	40.64	1.87	345	0.832
123	40.64	1.57	345	0.828
131	40.64	1.57	345	0.828
137	40.64	1.27	345	0.816
147	40.64	1.27	345	0.816
152	40.64	1.27	345	0.816
162	40.64	1.27	345	0.816
39	50.8	1.905	345	0.812
42	50.8	1.905	345	0.812
74	50.8	1.905	345	0.81
98	50.8	1.905	345	0.81

At area load of 6560 KN, there is no structure failure occurred as all the structural members and joints pass the unity check.

4.2.3. Deformation Pattern

The deflection of riser guard is also studied as figure below. The white dash line indicates the deformation of the riser guard under vessel impact load. In this case, maximum deformation occurred at the center of the riser guard.

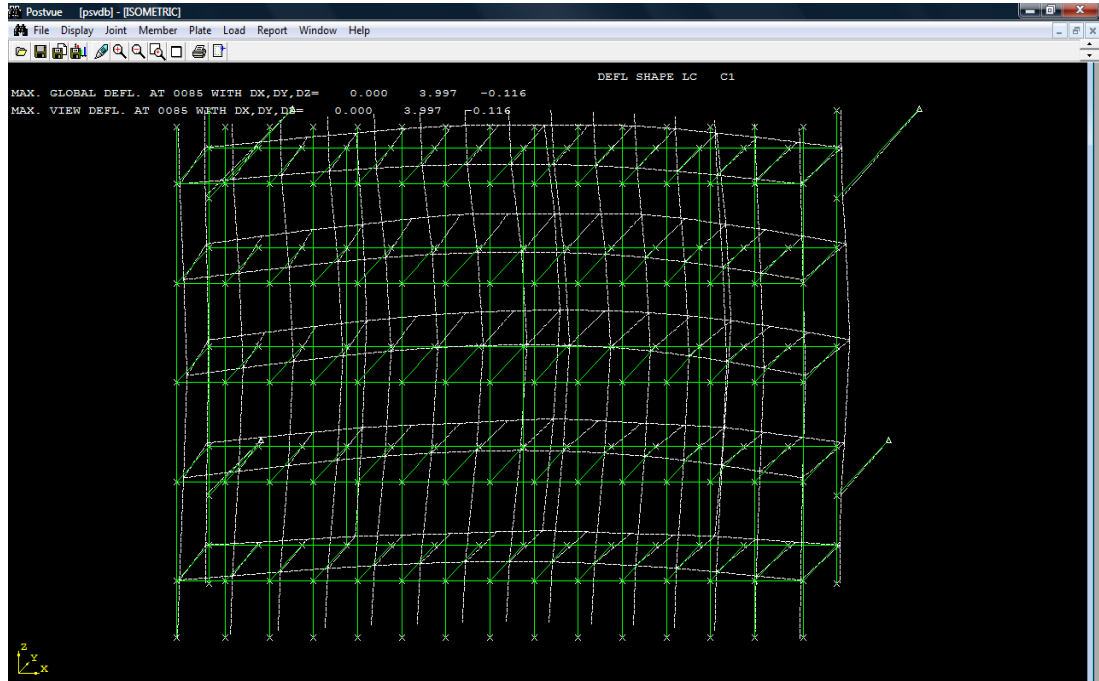


FIGURE 17: Deformation Pattern of Riser Guard

4.2.4. Structural Capacity

The riser guard is simulated under different magnitude of vessel impact loading to determine the actual capacity of the riser guard. The applied load is increased until it reaches the maximum load where the riser guard can take. Table below shows the applied load and its correspondent number of member and joint failed (Unity Check more than 1). The graph of area load versus number of structural component failed is plotted to determine the safe load for conventional riser guard. From the data, the maximum capacity of structural member is estimated to be 6560 KN.

TABLE 9: Area load applied with its correspondent number of structural component failed under vessel impact loading

Area Load (KN)	Number of member failed	Number of Joint failed
14000	122	102
12000	84	53
10000	38	25
8000	10	6
7000	2	0
6560	0	0

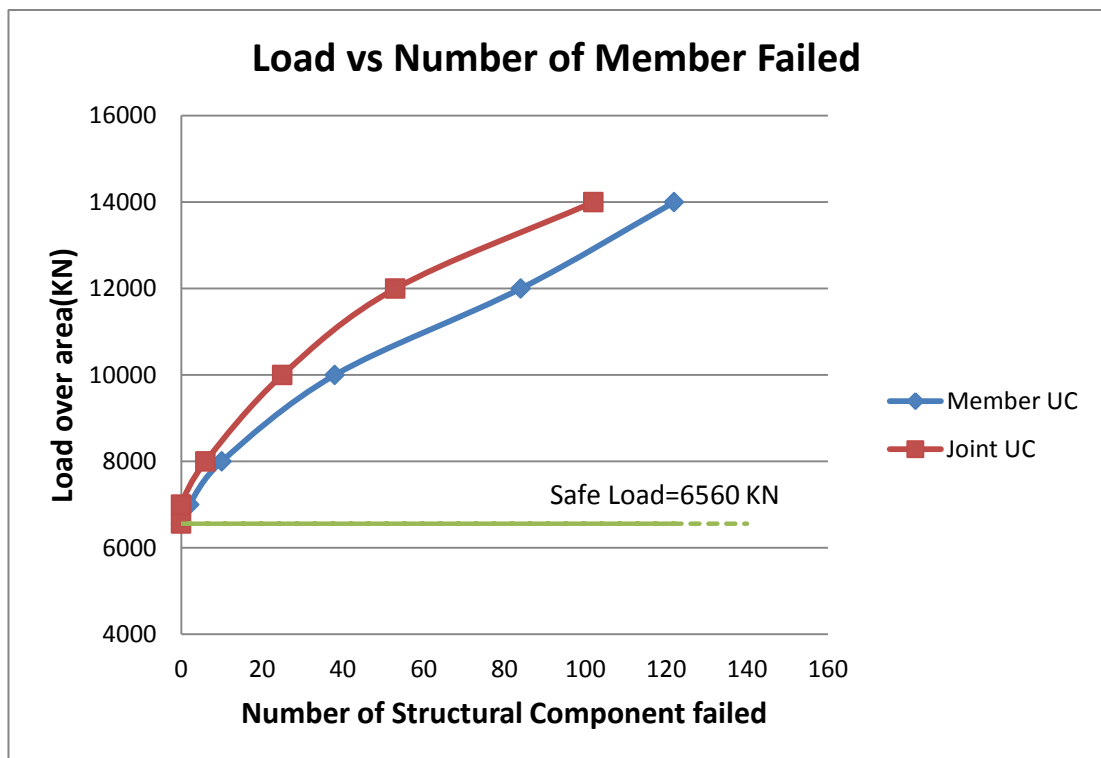


FIGURE 18: Graph of Area Load vs Number of Failed Structural Component

4.3. DYNAMIC ANALYSIS

The natural mode shape is generated from DYNPAC module after the master degree of freedom is identified. Its correspondent frequency, period and Eigenvalue are shown in Table 11.

TABLE 10: Natural Mode Shape

Mode	Freq.(cps)	Gen. mass	Eigenvalue	Period(secs)
1	11.766488	4.34E+01	1.83E-04	0.0849871
2	14.288871	7.76E+01	1.24E-04	0.0699845
3	19.71098	5.92E+01	6.52E-05	0.0507331
4	27.780892	5.02E+01	3.28E-05	0.035996
5	42.990555	3.32E+01	1.37E-05	0.0232609
6	55.4301	2.30E+01	8.24E-06	0.0180407

4.3.1 Extracted Mode Shape

The deformation of riser guard with six different mode shapes is illustrated as figure below. The white dashed line represents the deformation pattern of mode shape.

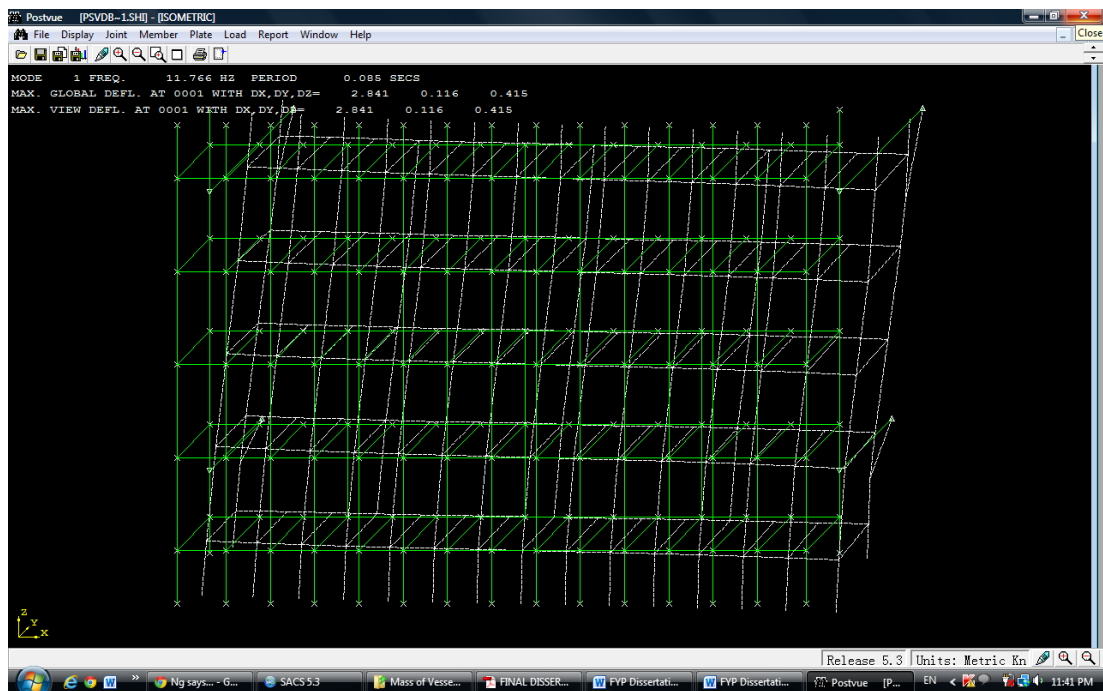


FIGURE 19: Deformation Pattern of Riser Guard for Mode 1

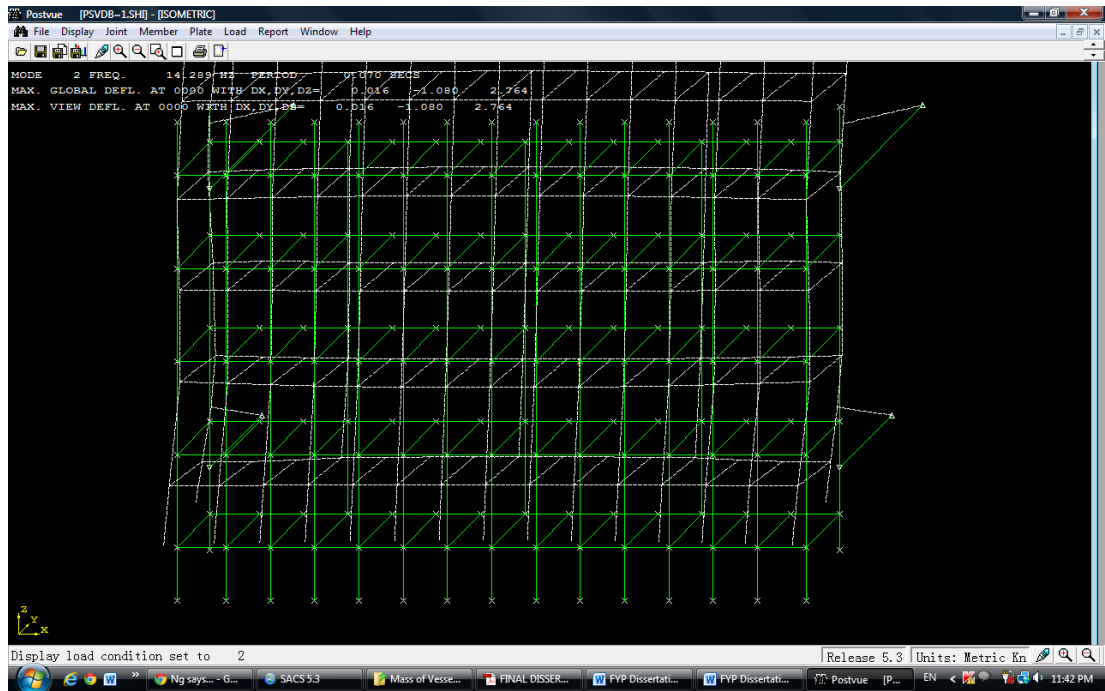


FIGURE 20: Deformation Pattern of Riser Guard for Mode 2

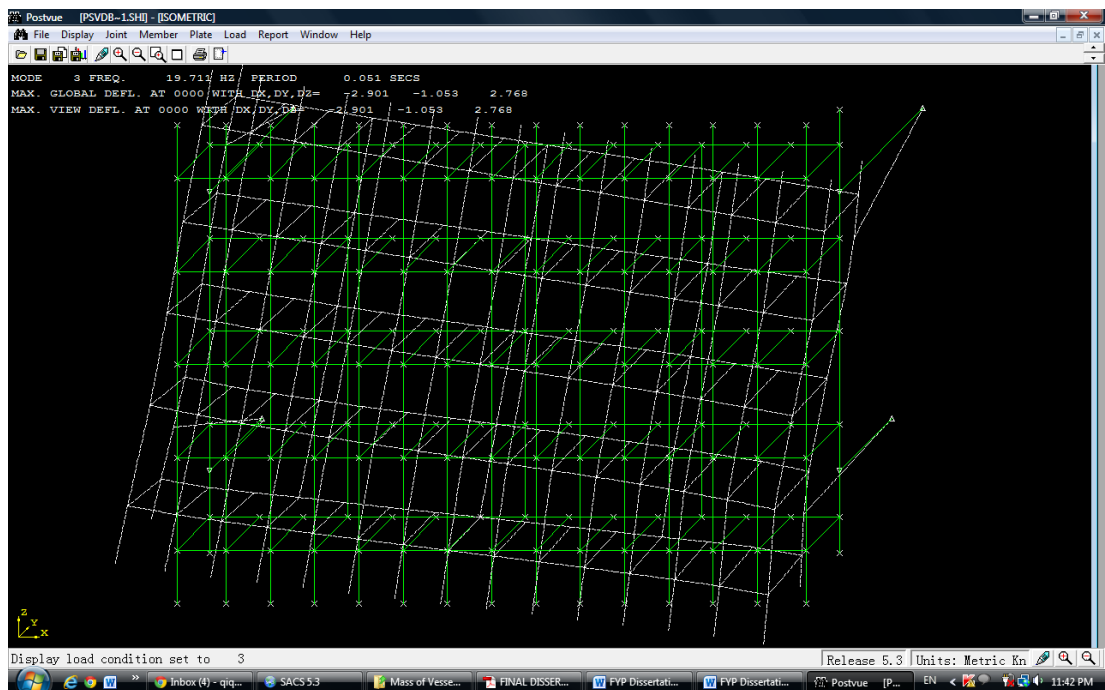


FIGURE 21: Deformation Pattern of Riser Guard for Mode 3

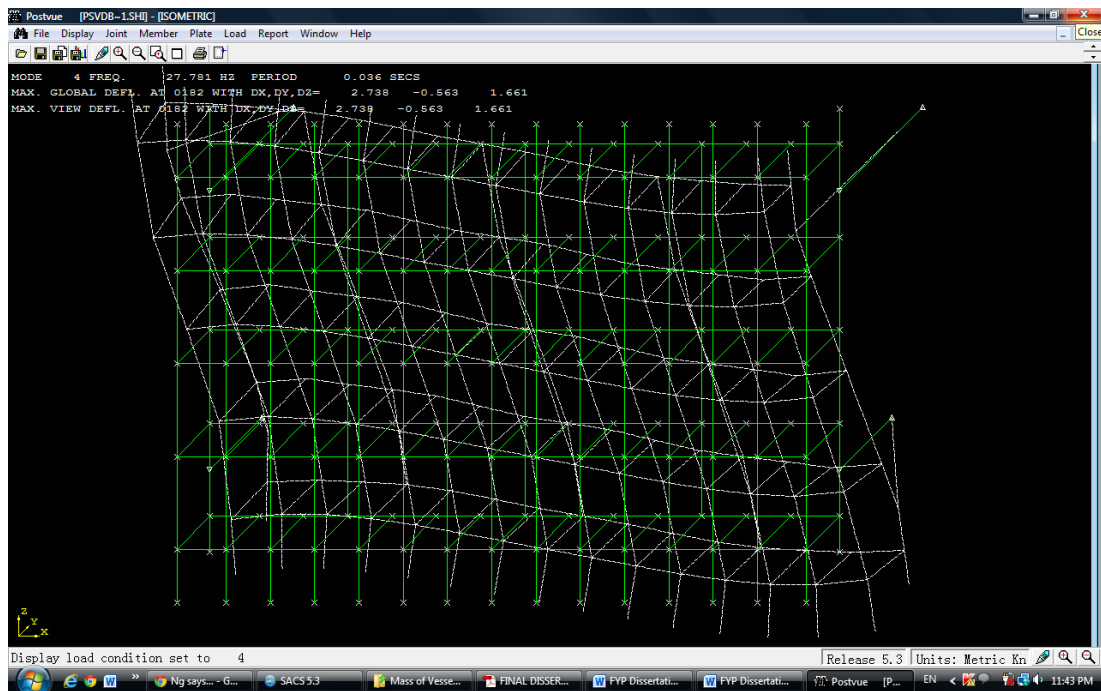


FIGURE 22: Deformation Pattern of Riser Guard for Mode 4

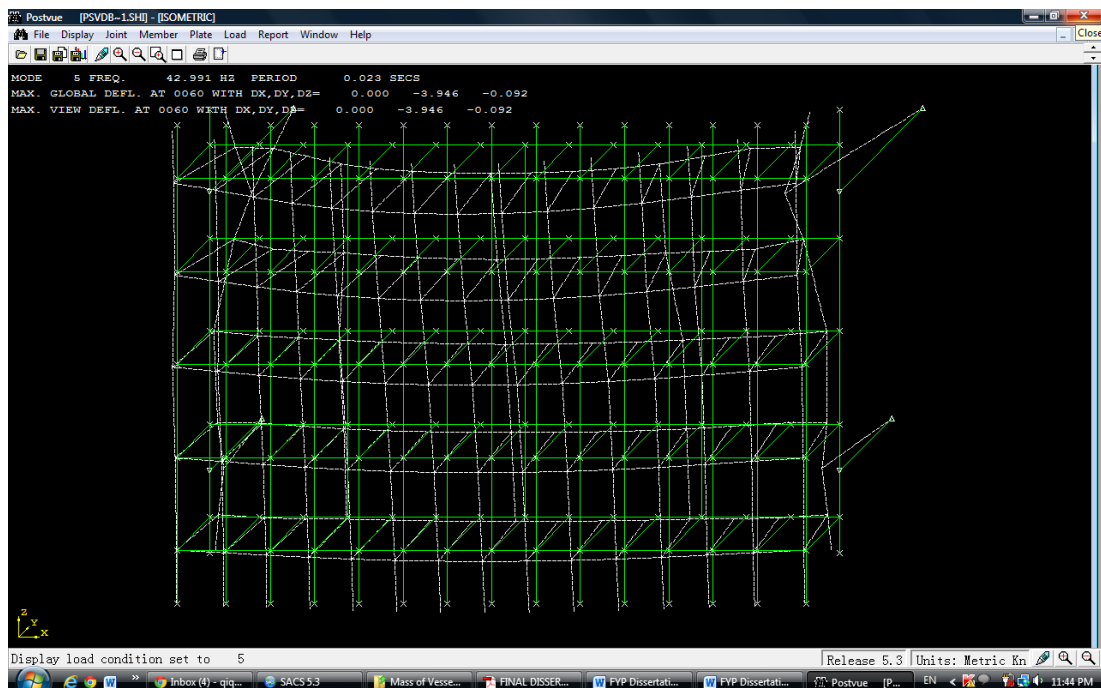


FIGURE 23: Deformation Pattern of Riser Guard for Mode 5

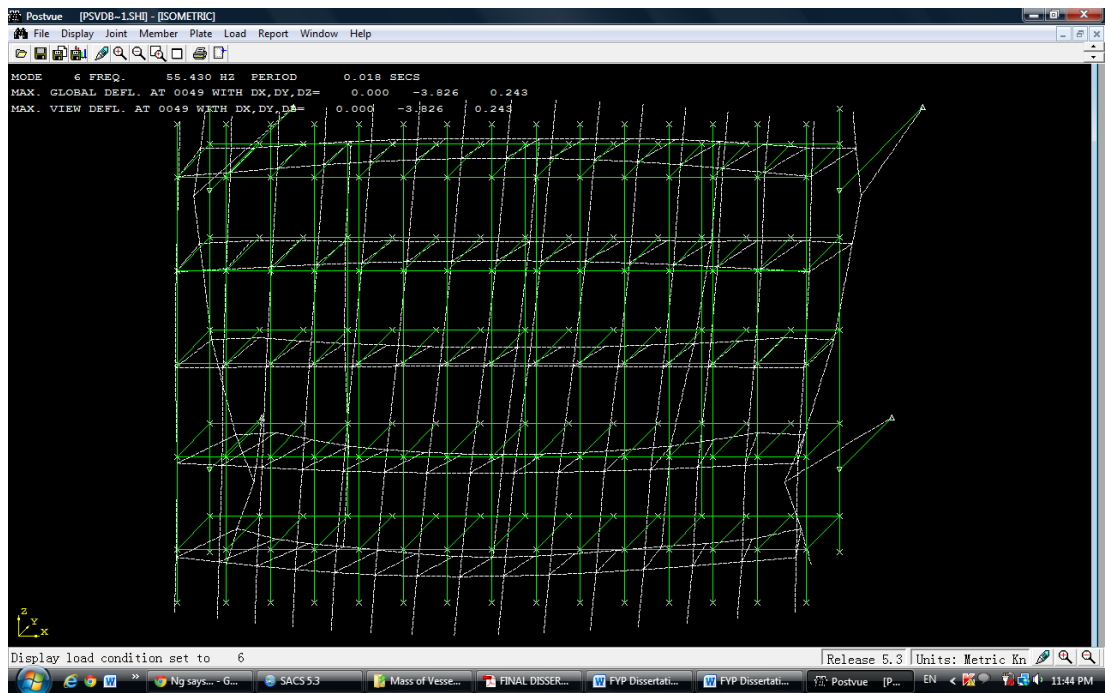


FIGURE 24: Deformation Pattern of Riser Guard for Mode 6

4.3.2. Parameter study

A parameter study of offshore riser guard under accidental vessel impact collisions was performed. My project will be mainly focused on two parameters which are mass of vessel and vessel impact velocity. Therefore, the discussion will be focused on the corresponding rate of deformation. Ship Impact Analysis and Collapse analysis were performed to study the deformation of riser guard upon vessel impact. The deformation of riser guard against broadside collision and stern/bow collision was tabulated in the table.

Stern/Bow Collision

Stern/Bow Collision			
Velocity (m/s)	Vessel Mass (Tonnes)	Ship Impact Force (KN)	Total displacement (cm)
0.5	2500	3745.47	68.439
1.0	3000	7504.76	73.137
2.5	4500	18803	96.211

Broadside Collision

Broadside Collision			
Velocity (m/s)	Vessel Mass (Tonnes)	Ship Impact Force (KN)	Total displacement (cm)
0.5	2500	3737.44	77.211
1.0	3000	7491.33	84.839
2.5	4500	18780.57	102.148

The broadside vessel collision scenario was defined as the situation in which vessel strikes most significant impact on offshore riser guard as compare to stern/bow vessel impact. When the accidental vessel collision occurred, offshore riser guard transform ship kinetic energy into strain energy by undergoes global deformation. Broadside collision will induce higher ship impact energy as compared to stern/bow collision. This is due to its larger surface area which lead to larger added mass resulting from its motion in water, thus the applied force to the riser guard is higher.

4.3.3 Stern/Bow Impact

4.3.3.1. Mass Deformation

Figure 21 presents the mass deformation relationship for different vessel mass and impact velocity. It can be seen that the offshore riser guard reach its maximum deformation at highest vessel mass. The force generate by the highest mass cause the riser guard to undergo large deformation which indicate that the riser guard induces large energy absorption. The offshore riser guard deforms 96.21 cm if it is hit by a 4500 tonnes vessel.

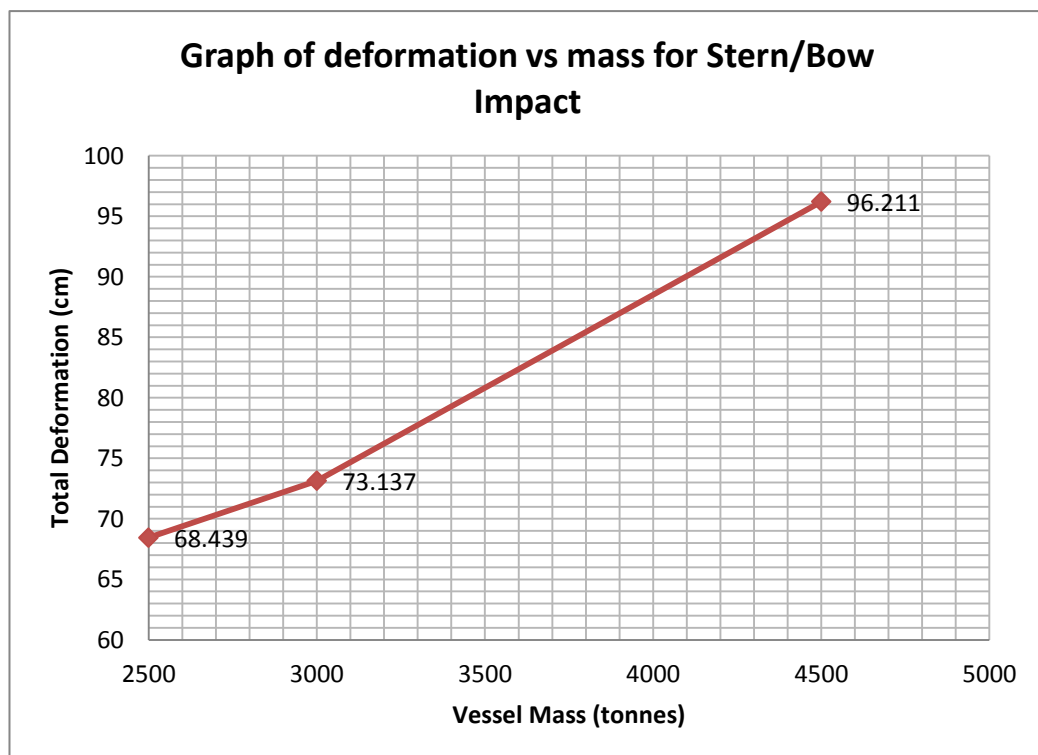


FIGURE 25: Mass-deformation (Stern/Bow Impact)

The design of riser guard is plasticity based where it allows the riser-guard to undergo large deformation for higher energy dissipation. The following figures show the plastic deformation of riser guard before collapse of the structure due to stern/bow collision impact at different vessel mass. Red colour members indicate that the member has reached its plasticity and plastic deformation has occurred. The load is applied to the structure incrementally. The nodal displacements and element forces are calculated for each load step and the stiffness matrix is updated. When the stress in a member reaches the yield stress plasticity is introduced. The introduction of plasticity reduces the stiffness of the structure and additional loads due to subsequent load increments will be redistributed to adjacent members to the members that have gone plastic. The structural behaviour of riser guard under vessel impact load before the structure collapse is illustrated in following figures:

4.3.3.2. Structural behaviour of riser guard

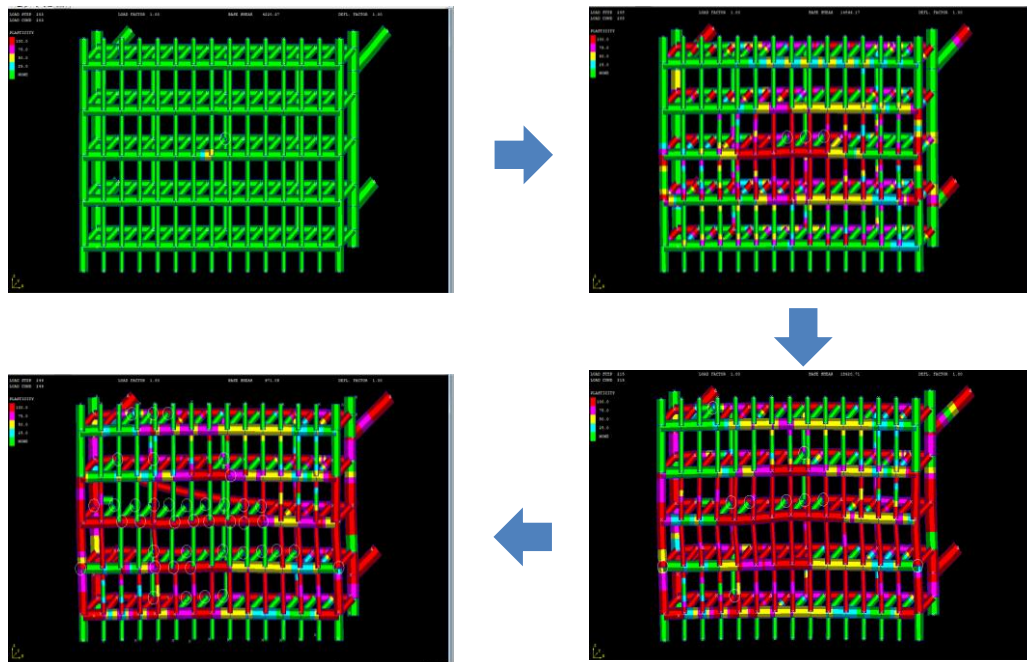


FIGURE 26: Structural behaviour of riser guard at 2500 tonnes (stern/bow impact)

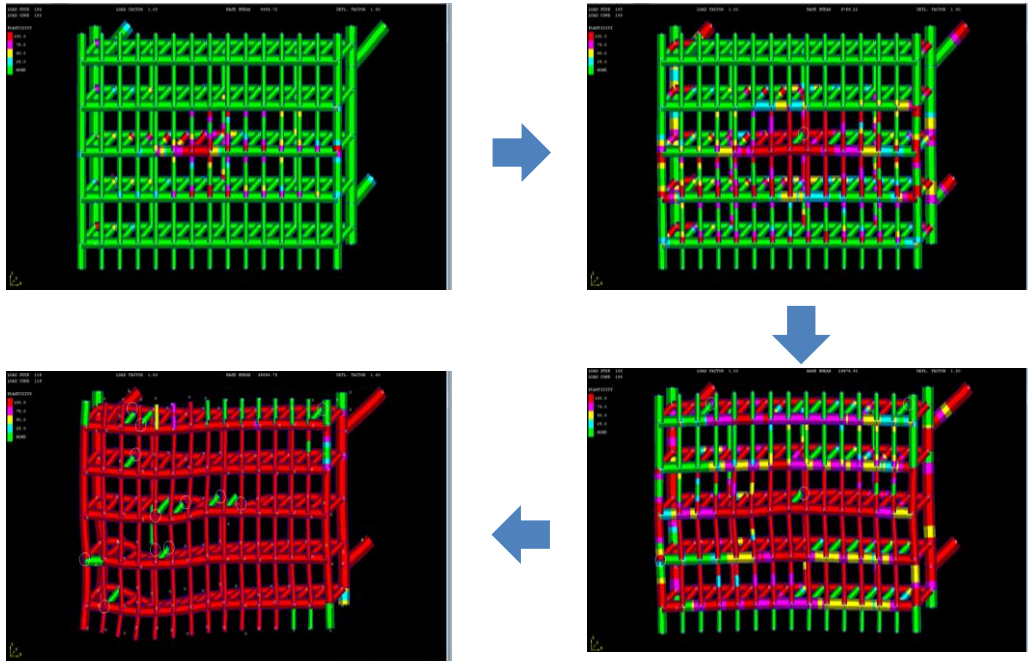


FIGURE 27: Structural behaviour of riser guard at 3000 tonnes (stern/bow impact)

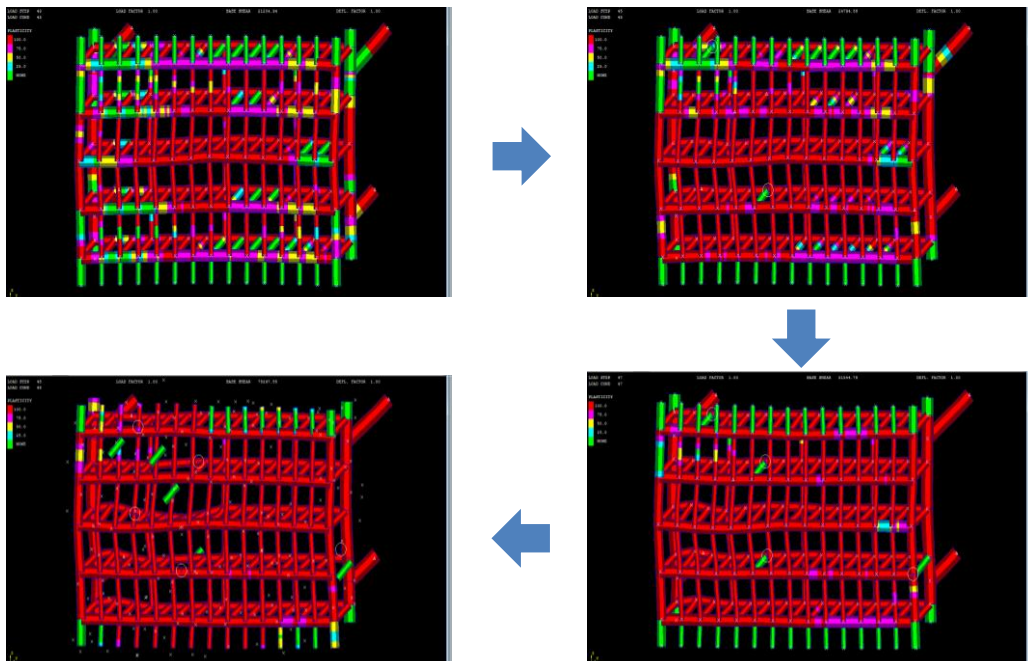


FIGURE 28: Structural behaviour of riser guard at 4500 tonnes (stern/bow impact)

These figures clearly show that number of member undergoes plastic deformation increases as the impact vessel mass increases. As the impact vessel mass and impact velocity increases, the impact force is increases. Thus number of member undergoes plastic deformation is increased for energy dissipation.

4.3.4. Broadside Impact

4.3.4.1. Mass Deformation

The relationship between the deformation and impact vessel mass were established by performing the collapse analysis. The following graph shows the deformation of riser guard at various impact vessel mass for broadside collision. From Figure 25, it can be seen that the maximum deformation occurred at vessel mass of 4500 tonnes. The offshore riser guard deforms 102.148cm if it is hit by a 4500 tonnes vessel. This indicates that the riser guard will undergo larger deformation if broadside collision occurred.

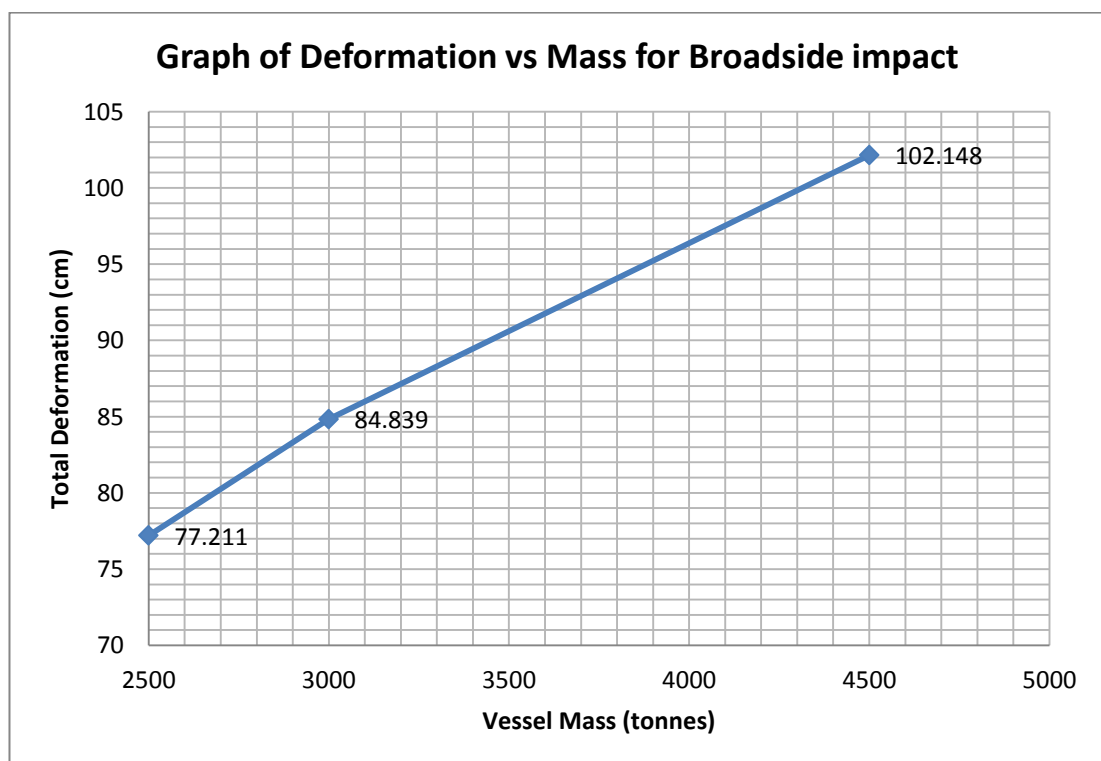


FIGURE 29: Mass-deformation (Broadside Impact)

The collapse analysis was performed to study the non-linear structural behaviour of offshore riser guard under different impact loading. The results obtained were presented in COLLVUE module in SACS. The COLLVUE modules below show the plastic deformation of riser guard and its plasticity under impact loading with various vessel mass for broadside impact collision. Red colour members indicate that the member has reached its plasticity and plastic deformation has occurred. When the stress in a member reaches the yield stress plasticity, stiffness of the structure and additional loads is reduced. Additional load due to subsequent load increments will be redistributed to adjacent members to the members that have gone plastic as illustrated in following figures.

4.3.4.2. Structural behaviour of riser guard

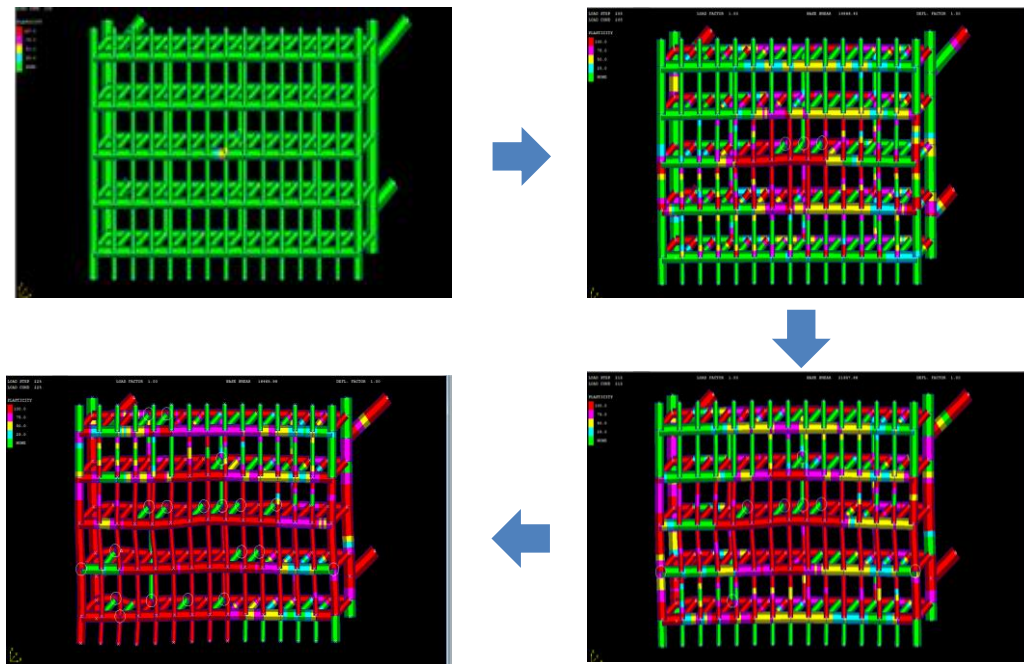


FIGURE 30: Structural behaviour of riser guard at 2500 tonnes (broadside impact)

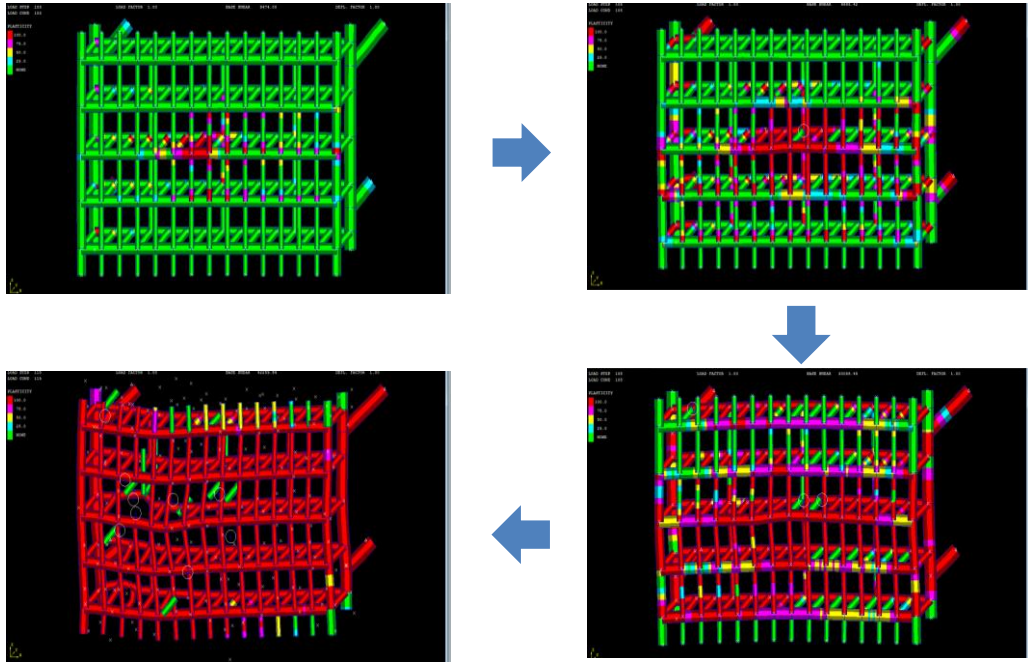


FIGURE 31: Structural behaviour of riser guard at 3000 tonnes (broadside impact)

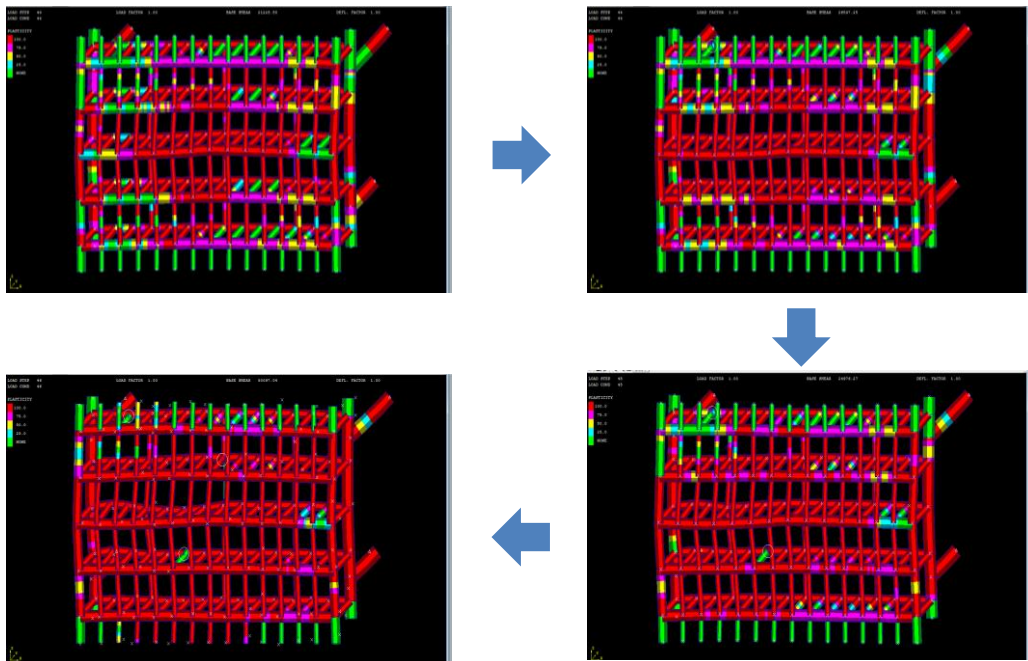


FIGURE 32: Structural behaviour of riser guard at 4500 tonnes (broadside impact)

In summary, these results show that:

- i. As vessel mass increases, the ship impact force increases and hence riser guard will undergo larger deformation to dissipate the vessel kinetic energy.
- ii. There is a significant increase in riser guard deformation when the impact velocity increases.
- iii. As the load is applied to the structure incrementally, the nodal displacements and element forces are calculated for each load step. When the stress in a member reaches the yield stress plasticity is introduced. The introduction of plasticity reduces the stiffness of the structure and additional loads due to subsequent load increments will be redistributed to adjacent members to the members that have gone plastic.

4.3.5. Damage assessment of riser guard under different vessel collision scenario

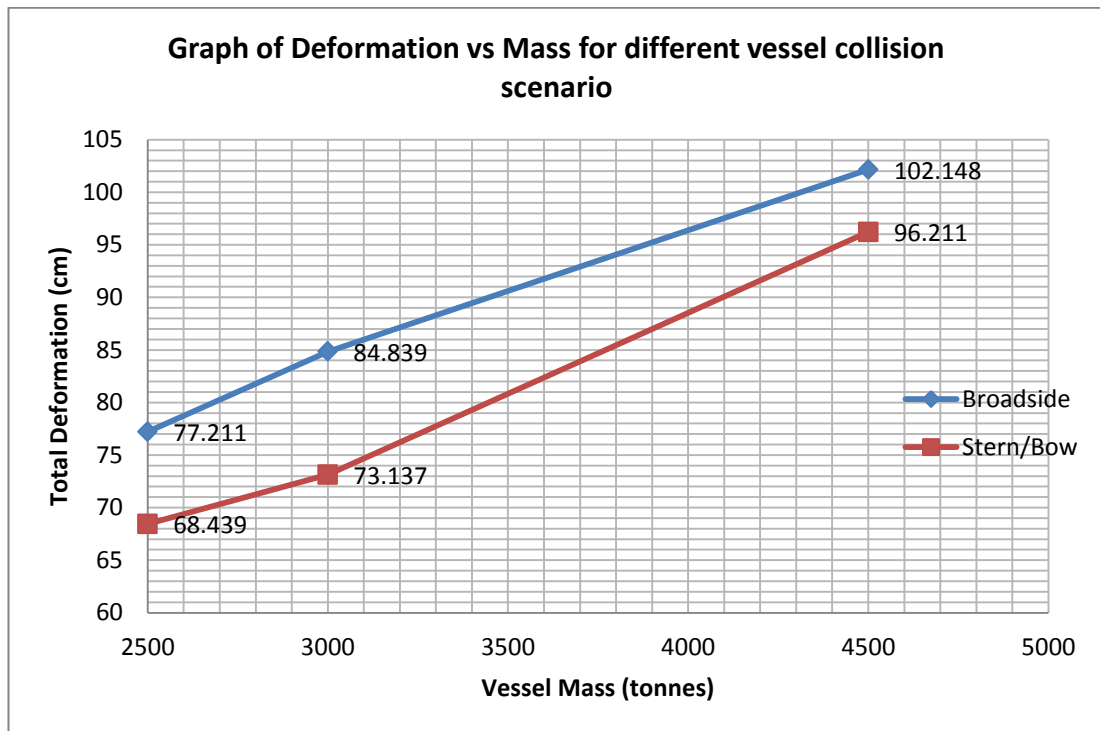


FIGURE 33: Mass-Deformation comparison of both scenarios

During impact, the kinetic of impacting vessel will partly remain as kinetic energy and partly dissipated as strain energy by riser-guard. Figure 29 show that broadside collision scenario will result in larger deformation of offshore riser guard. For broadside vessel collision, the ship impact force is higher due to its larger added mass resulting from its larger motion in water. In order to protect the risers against

vessel collision, riser guard undergo larger deformation for higher energy dissipation and reduction in impact force. Stern/bow vessel collision has lower ship impact force and thus smaller deformation of riser guard occurred.

As per conclusion based on simulation based on all the scenarios, broadside collision impact will cause more severe damage to the riser guard as compared to stern/bow collision. As the impact velocity and vessel mass increases, the resulted deformation increases for both the broadside and stern/bow side collision. The structural capacity of offshore riser guard is 6560 KN.

CHAPTER 5

CONCLUSION AND RECOMMENDATIONS

Damage assessment of conventional riser guard is important to understand the performance and structural behavior of the riser guard. The capability of the Structural Analysis Computer Software (SACS) was explored and the static-in-place analysis of conventional riser guard under loading equivalent to vessel collision is carried out. The unity check and deformation pattern of riser guard was analyzed. The results obtained shows that the conventional riser guard can take up to 6560 KN of load before the member failed. The ship impact analysis of riser guard was also carried out to understand ship impact force and also to generate the equivalent static load for the simulation of collapse analysis. Furthermore, plastic collapse analysis of offshore riser guard was performed to study the linear and non-linear behavior of riser guards, local deformation of impacted member due to beam bending and global deformation of the riser guards. During impact, the kinetic of impacting vessel will partly remain as kinetic energy and partly dissipated as strain energy by riser-guard. In order to protect the risers against vessel collision, riser guard undergo larger deformation for higher energy dissipation and reduction in impact force. The structural behaviour of offshore riser guard under different vessel collision scenarios were studied and results shows that deformation increases if vessel mass and impact velocity increases. Riser guard will undergo larger deformation if broadside collision occurred. In future, further research can be done to study the localised deformation of offshore riser guard under accidental impact loading. During ship collision occurred, local denting can occurred at structure and cause localised deformation. Hence, further research can be done on localised deformation for better understanding on structural behaviour of offshore riser guard.

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APPENDICES

A. SACS Input File (SACINP)

SACS Data Generator - [C:_sacinp.ship impact]									
OPTIONS	MN	DY	SDUC	1	1	DC	APT	PTPTPTPTPTPTPTPT	
GROUP									
GROUP 1		66.040	3.175	20.007	72424.80	1	1.001.00	0.500	7.8490
GROUP 2		27.300	1.270	20.007	72424.80	1	1.001.00	0.500	7.8490
GROUP 3		40.640	1.270	20.007	72424.80	1	1.001.00	0.500	7.8490
GROUP 4		50.800	1.905	20.007	72424.80	1	1.001.00	0.500	7.8490
GROUP 5		35.560	1.587	20.007	72424.80	1	1.001.00	0.500	7.8490
MEMBER									
MEMBER 01050182	1								
MEMBER 01190183	1								
MEMBER 01200186	1								
MEMBER 01340184	1								
MEMBER 01350150	1								
MEMBER 01490164	1								
MEMBER 01500165	1								
MEMBER 01640179	1								
MEMBER 01650187	1								
MEMBER 01790185	1								
MEMBER 01800120	1								
MEMBER 01810134	1								
MEMBER 01840149	1								
MEMBER 01840188	1								
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MEMBER 01850189	1								
MEMBER 01860135	1								
MEMBER 01860190	1								
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MEMBER 00050012	2								
MEMBER 00060013	2								
MEMBER 00120066	2								
MEMBER 00130102	2								
MEMBER 00140004	2								
MEMBER 00150007	2								
MEMBER 00160069	2								
MEMBER 00170072	2								
MEMBER 00180075	2								
MEMBER 00190078	2								
MEMBER 00200081	2								
MEMBER 00210084	2								
MEMBER 00220087	2								

SACS Data Generator - [C:_sacinp.ship impact]									
JOINT									
JOINT 0151	1.	1.	5.	10.800	50.000	40.000			
JOINT 0152	2.	1.	5.	10.800	50.000	40.000			
JOINT 0153	3.	1.	5.	10.800	50.000	40.000			
JOINT 0154	4.	1.	5.	10.800	50.000	40.000			
JOINT 0155	5.	1.	5.	10.800	50.000	40.000			
JOINT 0156	6.	1.	5.	10.800	50.000	40.000			
JOINT 0157	7.	1.	5.	10.800	50.000	40.000			
JOINT 0158	8.	1.	5.	10.800	50.000	40.000			
JOINT 0159	9.	1.	5.	10.800	50.000	40.000			
JOINT 0160	10.	1.	5.	10.800	50.000	40.000			
JOINT 0161	11.	1.	5.	10.800	50.000	40.000			
JOINT 0162	12.	1.	5.	10.800	50.000	40.000			
JOINT 0163	13.	1.	5.	10.800	50.000	40.000			
JOINT 0164	14.	1.	5.	21.600	50.000	40.000			
JOINT 0165	0.	1.	3.		50.000	30.000			
JOINT 0166	1.	1.	3.	10.800	50.000	30.000			
JOINT 0167	2.	1.	3.	10.800	50.000	30.000			
JOINT 0168	3.	1.	3.	10.800	50.000	30.000			
JOINT 0169	4.	1.	3.	10.800	50.000	30.000			
JOINT 0170	5.	1.	3.	10.800	50.000	30.000			
JOINT 0171	6.	1.	3.	10.800	50.000	30.000			
JOINT 0172	7.	1.	3.	10.800	50.000	30.000			
JOINT 0173	8.	1.	3.	10.800	50.000	30.000			
JOINT 0174	9.	1.	3.	10.800	50.000	30.000			
JOINT 0175	10.	1.	3.	10.800	50.000	30.000			
JOINT 0176	11.	1.	3.	10.800	50.000	30.000			
JOINT 0177	12.	1.	3.	10.800	50.000	30.000			
JOINT 0178	13.	1.	3.	10.800	50.000	30.000			
JOINT 0179	14.	1.	3.	21.600	50.000	30.000			
JOINT 0180	0.	1.	10.		50.000	40.000			
JOINT 0181	14.	1.	10.	21.600	50.000	40.000			
JOINT 0182	0.	1.	0.		50.000	40.000			
JOINT 0183	14.	1.	0.	21.600	50.000	40.000			
JOINT 0184	14.	1.	8.	21.600	50.000	55.000	222000		
JOINT 0185	14.	1.	2.	21.600	50.000	25.000	222000		
JOINT 0186	0.	1.	8.		50.000	55.000	222000		
JOINT 0187	0.	1.	2.		50.000	25.000	222000		
JOINT 0188	14.	5.	8.	21.600	27.500	55.000	111111		
JOINT 0189	14.	3.	2.	21.600	86.000	25.000	111111		
JOINT 0190	0.	5.	8.		27.500	55.000	111111		
JOINT 0191	0.	3.	2.		86.000	25.000	111111		

B. Dynamic Response Input File (DYRINP)

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PROPT	SHIP	EC+Z							
SDAMP		5.							
LOAD									
SHIP	2500.0000	0.5	180.	1.			0085	1.4	
THLOAD	SHIP	SDO		PLTPLMPLSPRTALLMXSCLPJPD					
JTNUM	0008000900100011003800400041004300850157016401880189								
TIME		5.0000	0.01	1.0E-9	1.00				
END									

C. Collapse Input File (CLPINP)

```
SACS Data Generator - [C:\...\clpinp.ship impact]
File Edit Search Tools Window Help

CLPOPT 20 8 20 CN JS SF 0.010.001 0.01 0.002
CLPRPT PIRIMIMP JISMMSPW ES
LDAPL DEAD SW 1 1.0
END
```

D. DYNPAC Output File (DYNLST)

```
Programmer's File Editor - [dynlst.shipimpact]
File Edit Options Template Execute Macro Window Help

SACS IU-FREQUENCIES AND GENERALIZED MASS

MODE      FREQ.(CPS)  GEN. MASS  EIGENVALUE  PERIOD(SECS)

1          11.766488  4.3382421E+01  1.8295594E-04  0.0849871
2          14.288871  7.7563965E+01  1.2406360E-04  0.0699845
3          19.710980  5.9211198E+01  6.5196426E-05  0.0507331
4          27.780892  5.0239880E+01  3.2820702E-05  0.0359960
5          42.990555  3.3232247E+01  1.3705476E-05  0.0232609
6          55.430100  2.3040410E+01  8.2442076E-06  0.0180407

SACS Release 5.3      urs      ID=00000000
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SACS IU MODE SHAPES

JOINT D.O.F.  MODE 1  MODE 2  MODE 3  MODE 4  MODE 5  MODE 6

0184 DIS X  1.000000  0.037207  0.334087  -0.743175  -1.000000  0.383330
0184 DIS Y  0.000869  0.032004  0.026711  -0.014920  -0.274264  0.105419
0184 DIS Z  -0.126753  1.000000  -1.000000  -0.685640  0.098307  -0.158741
0185 DIS X  0.460410  -0.025402  -0.792205  1.000000  -0.325621  -1.000000
0185 DIS Y  0.005511  -0.018852  0.011683  0.012131  -0.077731  -0.219525
0185 DIS Z  -0.121922  0.959101  -0.957683  -0.656412  0.088332  -0.137486
0186 DIS X  1.000000  -0.037207  0.334087  -0.743175  1.000000  -0.383330
0186 DIS Y  -0.000869  0.032004  0.026711  0.014920  -0.274264  0.105419
0186 DIS Z  0.126753  1.000000  1.000000  0.685640  0.098307  -0.158741
0187 DIS X  0.460410  0.025402  -0.792205  1.000000  0.325621  1.000000
0187 DIS Y  -0.005511  -0.018852  -0.011683  -0.012131  -0.077731  -0.219525
0187 DIS Z  0.121922  0.959101  0.957683  0.656411  0.088332  -0.137486
```

```
Programmer's File Editor - [dynlst.shipimpact]
File Edit Options Template Execute Macro Window Help

SACS Release 5.3      urs      ID=00000000
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***** MODAL REACTION SUMMARY *****
***** MOMENTS ABOUT ORIGIN *****

MODE  FORCE-X  FORCE-Y  FORCE-Z  MOMENT-X  MOMENT-Y  MOMENT-Z
      KN      KN      KN      KN-M      KN-M      KN-M
1      -7485.128  0.000  0.000  5478.3  -32677.7  -7316.9
2      0.000  -311.956  -16006.317  1854.7  113398.7  -2121.6
3      6738.725  0.000  0.000  9695.7  -113645.2  2179.9
4      -7620.457  0.000  0.000  11348.9  -79156.6  4221.1
5      0.000  67269.520  -2252.152  -222433.4  39268.6  462097.1
6      0.000  41470.124  3567.506  19576.6  -18024.9  285243.5

SACS Release 5.3      urs      ID=00000000
DATE 08-AUG-2014  TIME 18:16:24  DYN PAGE 17

BASE SHEAR AND OVERTURNING MOMENT COEFFICIENTS

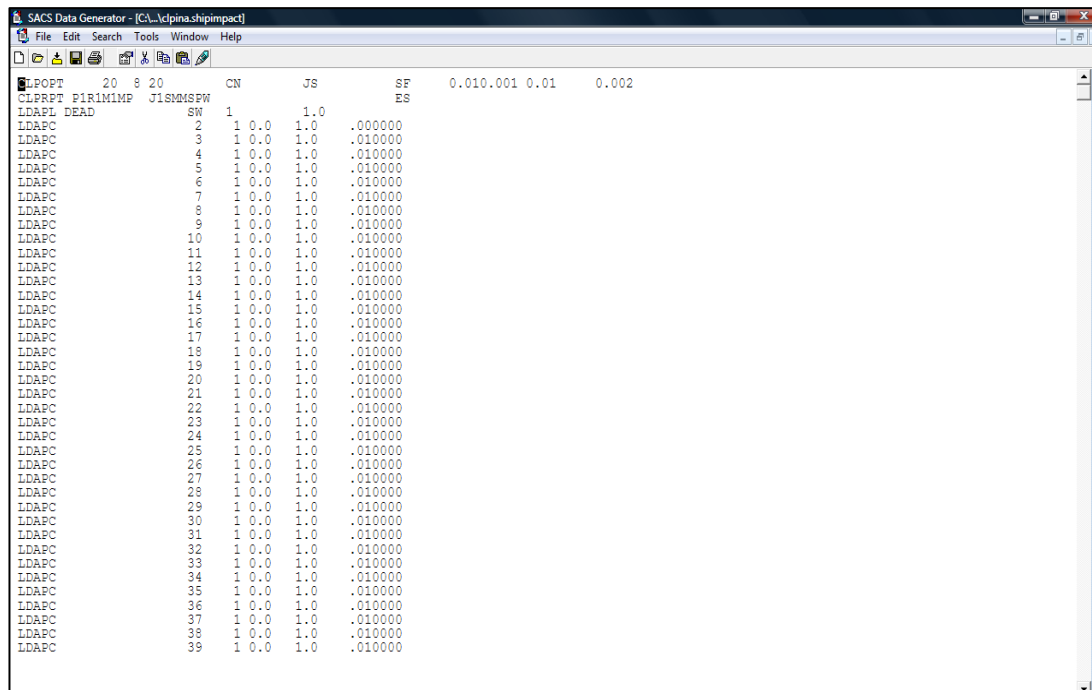
MODE  SHEAR(X)  SHEAR(Y)  MOMENT(X)  MOMENT(Y)
      KN      KN      KN-M      KN-M
1      -7485.128  0.000  5478.326  -32677.720
2      0.000  -311.956  1854.681  113398.686
3      6738.725  0.000  9695.734  -113645.194
4      -7620.457  0.000  11348.862  -79156.545
5      0.000  67269.520  -222433.331  39268.546
6      0.000  41470.124  19576.625  -18024.937

SACS Release 5.3      urs      ID=00000000
DATE 08-AUG-2014  TIME 18:16:24  DYN PAGE 18

BASE DRIVING INFLUENCE MATRIX

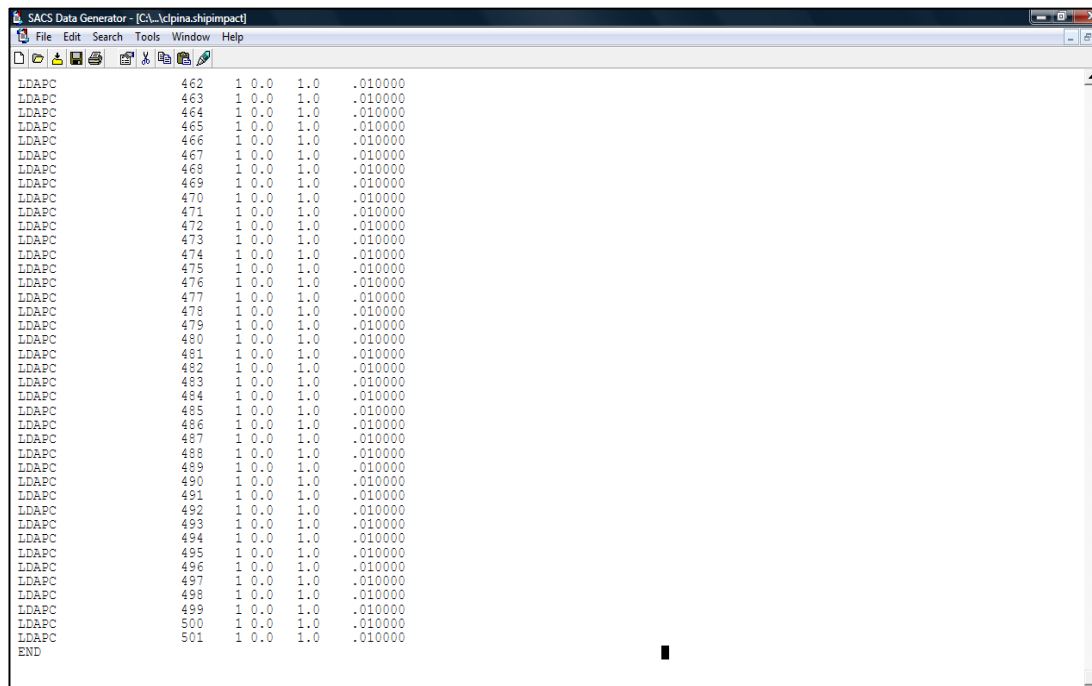
MODE  X-DIRECTION  Y-DIRECTION  Z-DIRECTION
```

E. Collapse Dynamic Loading Input File (CLPINA)



The screenshot shows the SACS Data Generator window with the following data:

EPOPT		20	8	20	CN	JS	SF	0.010.001 0.01 0.002	
CLPRPT	PIRIMIMP	JISMMSPW					ES		
LDAPL	DEAD	SW	1		1.0				
LDAPC		2	1	0.0	1.0		.000000		
LDAPC		3	1	0.0	1.0		.010000		
LDAPC		4	1	0.0	1.0		.010000		
LDAPC		5	1	0.0	1.0		.010000		
LDAPC		6	1	0.0	1.0		.010000		
LDAPC		7	1	0.0	1.0		.010000		
LDAPC		8	1	0.0	1.0		.010000		
LDAPC		9	1	0.0	1.0		.010000		
LDAPC		10	1	0.0	1.0		.010000		
LDAPC		11	1	0.0	1.0		.010000		
LDAPC		12	1	0.0	1.0		.010000		
LDAPC		13	1	0.0	1.0		.010000		
LDAPC		14	1	0.0	1.0		.010000		
LDAPC		15	1	0.0	1.0		.010000		
LDAPC		16	1	0.0	1.0		.010000		
LDAPC		17	1	0.0	1.0		.010000		
LDAPC		18	1	0.0	1.0		.010000		
LDAPC		19	1	0.0	1.0		.010000		
LDAPC		20	1	0.0	1.0		.010000		
LDAPC		21	1	0.0	1.0		.010000		
LDAPC		22	1	0.0	1.0		.010000		
LDAPC		23	1	0.0	1.0		.010000		
LDAPC		24	1	0.0	1.0		.010000		
LDAPC		25	1	0.0	1.0		.010000		
LDAPC		26	1	0.0	1.0		.010000		
LDAPC		27	1	0.0	1.0		.010000		
LDAPC		28	1	0.0	1.0		.010000		
LDAPC		29	1	0.0	1.0		.010000		
LDAPC		30	1	0.0	1.0		.010000		
LDAPC		31	1	0.0	1.0		.010000		
LDAPC		32	1	0.0	1.0		.010000		
LDAPC		33	1	0.0	1.0		.010000		
LDAPC		34	1	0.0	1.0		.010000		
LDAPC		35	1	0.0	1.0		.010000		
LDAPC		36	1	0.0	1.0		.010000		
LDAPC		37	1	0.0	1.0		.010000		
LDAPC		38	1	0.0	1.0		.010000		
LDAPC		39	1	0.0	1.0		.010000		



The screenshot shows the SACS Data Generator window with the following data:

LDAPC		462	1	0.0	1.0		.010000		
LDAPC		463	1	0.0	1.0		.010000		
LDAPC		464	1	0.0	1.0		.010000		
LDAPC		465	1	0.0	1.0		.010000		
LDAPC		466	1	0.0	1.0		.010000		
LDAPC		467	1	0.0	1.0		.010000		
LDAPC		468	1	0.0	1.0		.010000		
LDAPC		469	1	0.0	1.0		.010000		
LDAPC		470	1	0.0	1.0		.010000		
LDAPC		471	1	0.0	1.0		.010000		
LDAPC		472	1	0.0	1.0		.010000		
LDAPC		473	1	0.0	1.0		.010000		
LDAPC		474	1	0.0	1.0		.010000		
LDAPC		475	1	0.0	1.0		.010000		
LDAPC		476	1	0.0	1.0		.010000		
LDAPC		477	1	0.0	1.0		.010000		
LDAPC		478	1	0.0	1.0		.010000		
LDAPC		479	1	0.0	1.0		.010000		
LDAPC		480	1	0.0	1.0		.010000		
LDAPC		481	1	0.0	1.0		.010000		
LDAPC		482	1	0.0	1.0		.010000		
LDAPC		483	1	0.0	1.0		.010000		
LDAPC		484	1	0.0	1.0		.010000		
LDAPC		485	1	0.0	1.0		.010000		
LDAPC		486	1	0.0	1.0		.010000		
LDAPC		487	1	0.0	1.0		.010000		
LDAPC		488	1	0.0	1.0		.010000		
LDAPC		489	1	0.0	1.0		.010000		
LDAPC		490	1	0.0	1.0		.010000		
LDAPC		491	1	0.0	1.0		.010000		
LDAPC		492	1	0.0	1.0		.010000		
LDAPC		493	1	0.0	1.0		.010000		
LDAPC		494	1	0.0	1.0		.010000		
LDAPC		495	1	0.0	1.0		.010000		
LDAPC		496	1	0.0	1.0		.010000		
LDAPC		497	1	0.0	1.0		.010000		
LDAPC		498	1	0.0	1.0		.010000		
LDAPC		499	1	0.0	1.0		.010000		
LDAPC		500	1	0.0	1.0		.010000		
LDAPC		501	1	0.0	1.0		.010000		
END									

F. Collapse History Log File (CLPLOG)

Programmer's File Editor - (clplog.shipimpact)

File Edit Options Template Execute Macro Window Help

**** NON-LINEAR COLLAPSE ANALYSIS (LOAD SEQUENCE 1) ****

NSLU	INC.	LOAD LOOP	CASE	LOAD FACTOR	*DEFLECTION* DIFF. JNT DOF	ROTATION DIFFERENCE	** DEFLECTION ** MAXIMUM JNT DOF	% OF IMPACT ENERGY
2	1	1	SW	1.000	0.0001 0049 DY	0.0000002	-0.302 0049 DZ	
2	1	2	SW	1.000	0.0000 0049 DZ	0.0000000	-0.302 0049 DZ	
4	2	1	2	1.000	0.0000 0060 DY	0.0000000	-0.302 0049 DZ	
4	2	2	2	1.000	0.0000 0049 DY	0.0000000	-0.302 0049 DZ	
6	3	1	3	1.000	0.0000 0044 DX	0.0000000	-0.302 0049 DZ	
6	3	2	3	1.000	0.0000 0060 DY	0.0000000	-0.302 0049 DZ	
8	4	1	4	1.000	0.0000 0125 DZ	0.0000000	-0.302 0049 DZ	
8	4	2	4	1.000	0.0000 0044 DX	0.0000000	-0.302 0049 DZ	
10	5	1	5	1.000	0.0000 0024 DZ	0.0000000	-0.302 0049 DZ	
10	5	2	5	1.000	0.0000 0125 DZ	0.0000000	-0.302 0049 DZ	
12	6	1	6	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
12	6	2	6	1.000	0.0000 0024 DZ	0.0000000	-0.302 0049 DZ	
14	7	1	7	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
14	7	2	7	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
16	8	1	8	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
16	8	2	8	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
18	9	1	9	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
18	9	2	9	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
20	10	1	10	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
20	10	2	10	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
22	11	1	11	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
22	11	2	11	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
24	12	1	12	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
24	12	2	12	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
26	13	1	13	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
26	13	2	13	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
28	14	1	14	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
28	14	2	14	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
30	15	1	15	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
30	15	2	15	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
32	16	1	16	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
32	16	2	16	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
34	17	1	17	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
34	17	2	17	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
36	18	1	18	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
36	18	2	18	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	

Programmer's File Editor - (clplog.shipimpact)

File Edit Options Template Execute Macro Window Help

390	195	1	195	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
390	195	2	195	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
392	196	1	196	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
392	196	2	196	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
394	197	1	197	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
394	197	2	197	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
396	198	1	198	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
396	198	2	198	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
398	199	1	199	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
398	199	2	199	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
400	200	1	200	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
400	200	2	200	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
402	201	1	201	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
402	201	2	201	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
404	202	1	202	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
404	202	2	202	1.000	0.0000 0191 DX	0.0000000	-0.302 0049 DZ	
406	203	1	203	1.000	0.0125 0085 DY	0.0000902	2.140 0085 DY	
407	203	2	203	1.000	0.0049 0085 DY	0.0000481	2.145 0085 DY	
*** WARNING - JOINT FAILURE OCCURRED AT JOINT 0157 FOR BRACE MEMBER 0142-0157 AT LOAD STEP 202								
408	203	1	203	1.000	0.2384 0142 DZ	0.0018766	2.296 0085 DY	
409	203	2	203	1.000	0.0073 0085 DY	0.0000481	2.304 0085 DY	
409	203	3	203	1.000	0.2384 0142 DZ	0.0018766	2.296 0085 DY	
411	204	1	204	1.000	0.0464 0118 DX	0.0001121	-1.487 0006 DX	
412	204	2	204	1.000	0.0178 0185 DX	0.0000476	-1.494 0006 DX	
413	204	3	204	1.000	0.0076 0003 DX	0.0000202	-1.497 0006 DX	
413	204	4	204	1.000	0.0178 0185 DX	0.0000476	-1.494 0006 DX	
415	205	1	205	1.000	0.1340 0085 DY	0.0004297	-2.546 0085 DX	
416	205	2	205	1.000	0.0733 0085 DY	0.0003130	-2.574 0085 DX	
417	205	3	205	1.000	0.0477 0085 DY	0.0002535	-2.589 0085 DX	
418	205	4	205	1.000	0.0342 0085 DY	0.0001953	-2.599 0085 DX	
419	205	5	205	1.000	0.0266 0085 DY	0.0001536	-2.606 0085 DX	
420	205	6	205	1.000	0.0222 0085 DY	0.0001570	-2.612 0085 DX	
421	205	7	205	1.000	0.0188 0085 DY	0.0001231	-2.617 0085 DX	
422	205	8	205	1.000	0.0161 0085 DY	0.0000987	-2.621 0085 DX	
423	205	9	205	1.000	0.0153 0085 DY	0.0000864	-2.625 0085 DX	
424	205	10	205	1.000	0.0137 0085 DY	0.0000714	-2.628 0085 DX	
425	205	11	205	1.000	0.0118 0085 DY	0.0000601	-2.631 0085 DX	
426	205	12	205	1.000	0.0103 0085 DY	0.0000517	-2.636 0085 DY	
427	205	13	205	1.000	0.0091 0085 DY	0.0000449	-2.645 0085 DY	
*** WARNING - JOINT FAILURE OCCURRED AT JOINT 0157 FOR BRACE MEMBER 0085-0157 AT LOAD STEP 204								
428	205	1	205	1.000	0.1875 0157 DY	0.0013452	2.760 0085 DY	

G. Dynamic Listing File (DYRLST)

```

Programmer's File Editor - [dplog.shipimpact]
File Edit Options Template Execute Macro Window Help

814 224 7 224 1.000 3.9340 0182 DY 0.0275428 -57.648 0085 DX
815 224 8 224 1.000 4.9868 0182 DY 0.0337387 -58.326 0085 DX
816 224 9 224 1.000 5.4098 0182 DY 0.0375179 -57.633 0085 DX
817 224 10 224 1.000 6.8858 0182 DY 0.0463624 -58.596 0085 DX
818 224 11 224 1.000 8.6429 0182 DY 0.0602400 -57.571 0085 DX
819 224 12 224 1.000 10.0980 0182 DY 0.0686208 -59.104 0085 DX
820 224 13 224 1.000 14.1593 0182 DY 0.0979955 -56.742 0085 DX
821 224 14 224 1.000 14.6580 0182 DY 0.1007888 -59.174 0085 DX
822 224 15 224 1.000 20.0506 0182 DY 0.1394834 -56.332 0085 DX
823 224 16 224 1.000 15.9666 0182 DY 0.1095334 -59.207 0085 DX
824 224 17 224 1.000 16.4294 0182 DY 0.1128230 -56.956 0085 DX
825 224 18 224 1.000 12.3949 0182 DY 0.0844341 -59.072 0085 DX
826 224 19 224 1.000 10.0266 0182 DY 0.0683247 -57.341 0085 DX
826 224 20 224 1.000 6.1975 0182 DY 0.0422170 -58.347 0085 DX
*** WARNING - JOINT FAILURE OCCURRED AT JOINT 0160 FOR BRACE MEMBER 0094-0160 AT LOAD STEP 224
828 225 1 225 1.000 17.2635 0182 DY 0.1192466 -58.819 0085 DX
829 225 2 225 1.000 80.8754 0182 DY 0.5537826 -52.489 0085 DX
830 225 3 225 1.000 4.5237 0182 DY 0.0740700 -57.691 0085 DX
831 225 4 225 1.000 2.2278 0108 DZ 0.0587329 -57.669 0085 DX
832 225 5 225 1.000 9.7081 0108 DZ 0.0821231 -57.817 0085 DX
833 225 6 225 1.000 2.6443 0045 DY 0.0608186 -57.867 0085 DX
834 225 7 225 1.000 12.6303 0108 DZ 0.1704164 -58.049 0085 DX
835 225 8 225 1.000 4.4855 0108 DZ 0.1155213 -58.022 0085 DX
836 225 9 225 1.000 8.4820 0045 DY 0.0530795 -57.783 0085 DX
837 225 10 225 1.000 10.2970 0108 DZ 0.1394541 -58.258 0085 DX
838 225 11 225 1.000 5.9201 0108 DZ 0.1323841 -58.032 0085 DX
839 225 12 225 1.000 17.0052 0045 DY 0.0838534 -57.792 0085 DX
840 225 13 225 1.000 10.3967 0108 DZ 0.1861992 -58.278 0085 DX
841 225 14 225 1.000 3.2302 0109 DZ 0.1307427 -58.167 0085 DX
842 225 15 225 1.000 5.5562 0109 DZ 0.1837132 -58.175 0085 DX
843 225 16 225 1.000 6.5864 0045 DY 0.0658671 -57.821 0085 DX
844 225 17 225 1.000 31.7808 0108 DZ 0.3978164 -58.757 0085 DX
845 225 18 225 1.000 59.2214 0045 DY 0.3242792 -56.206 0085 DX
846 225 19 225 1.000 225.9433 0045 DY 1.6761273 212.402 0045 DY
846 225 20 225 1.000 29.6107 0045 DY 0.1621396 -57.172 0085 DX
*** WARNING - JOINT FAILURE OCCURRED AT JOINT 0116 FOR BRACE MEMBER 0036-0116 AT LOAD STEP 225
848 226 1 226 1.000 862.7675 0047 DY 62.6648281 -7931.705 0047 DY
848 226 2 226 1.000 0.0000 0047 DY 0.0000000 -7931.705 0047 DY

```

```

Programmer's File Editor - [dyrlst.shipimpact]
File Edit Options Template Execute Macro Window Help

DATE 08-AUG-2014 TIME 18:26:37 CLP PAGE 4834

**** FINAL DEFLECTIONS AND ROTATIONS FOR LOAD CASE 226 ****

***** DEFLECTIONS ***** ***** ROTATIONS *****
JOINT X Y Z X Y Z
CM CM CM RAD RAD RAD

0000 47.474-1345.537 478.130 -7.32123 -0.12034 -1.29926
0001 -0.840 154.032 524.554 -0.55669 -0.61932 0.92183
0002 11.339 108.648 202.822 -0.41350 -0.23074 -0.93216
0003 844.847 -363.941 259.966 -1.31349 -0.75071 2.06606
0004 639.679-3699.100 -587.290 -25.78566 -2.38307 -6.16545
0005 -34.711 264.747 623.315 -0.57452 -0.92459 0.91301
0006 4.523 243.871 171.166 -0.58616 -0.29773 -0.98423
0007 769.183 -757.759 214.915 -2.13997 -0.29102 2.47636
0008 71.761 85.716 522.581 -0.55671 -0.61931 0.92183
0009 40.104 58.686 202.429 -0.41352 -0.23072 -0.93216
0010 17.029 -472.535 555.998 -7.26837 -0.11525 -1.28667
0011 756.166 -212.253 258.704 -1.31348 -0.75072 2.06606
0012 74.217 192.923 619.126 -0.57464 -0.92452 0.91302
0013 41.871 172.454 170.525 -0.58630 -0.29766 -0.98423
0014 158.796-1030.074 365.873 -18.34058 -1.57784 -5.79769
0015 738.555 -504.394 218.222 -2.13996 -0.29110 2.47636
0016 76.288 298.074 716.576 -0.31231 -0.16535 0.89812
0017 75.798 386.382 773.216 0.05067 -0.19425 0.90997
0018 73.085 470.959 753.954 0.24237 0.52808 0.71231
0019 72.518 527.016 658.586 0.30591 1.07128 0.40209
0020 71.502 555.815 522.723 0.38262 1.27274 0.17236
0021 69.685 561.872 385.666 0.37594 1.16374 -0.03045
0022 66.292 547.320 273.043 0.21188 0.92660 -0.21488
0023 61.670 514.314 190.707 -0.10477 0.63854 -0.39791
0024 56.314 461.399 141.283 -0.47361 0.33004 -0.60126
0025 50.657 385.411 124.800 -0.74231 -0.00139 -0.82244
0026 45.498 284.296 140.928 -0.71739 -0.23746 -0.98035
0027 329.130-1822.839 70.523 -28.58269 -2.12131 -6.25174
0028 458.148-2510.488 -268.849 -37.89418 -0.69623 -4.60519
0029 475.189-2751.321 -356.992 -39.39912 0.16773 -1.80768

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*** SACS COLLAPSE REACTION FORCES AND MOMENTS ***

*** FINAL ***

JOINT NO.	FORCE(X) KN	FORCE(Y) KN	FORCE(Z) KN	MOMENT(X) KN-M	MOMENT(Y) KN-M	MOMENT(Z) KN-M
0188	-230623.709	-586384.751	-76552.378	182939.950	22679.473	-343461.835
0189	-645124.358	653183.714	-89143.298	333856.318	196248.338	-1368469.900
0190	-106373.976	-1018241.568	-295815.057	558906.469	-27520.286	-54093.518
0191	993215.218	955836.208	461690.438	72157.390	-870039.315	1151653.413

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*** SACS COLLAPSE MEMBER FORCES AND MOMENTS ***

*** FINAL ***

MEMBER	GRP	LOC. H	***** INTERNAL FORCES *****			***** INTERNAL MOMENTS *****			***** STRESSES *****						PLAST. RATIO
			X KN	Y KN	Z KN	X KN-M	Y KN-M	Z KN-M	AXIAL N/MM2	BEND-Y N/MM2	BEND-Z N/MM2	SHEAR-Y N/MM2	SHEAR-Z N/MM2		
0105-0182	1	0.00	-3495.741	-10073.594	2867.870	3328.1	1428.6	866.8	-55.7	151.9	92.2	-321.3	91.5	1.00	
		0.10	-3495.741	-10073.594	2867.870	3328.1	1749.9	-120.4	-55.7	186.1	-12.8	-321.3	91.5	1.00	
		0.20	-89.863	-3256.786	-3813.950	-24.2	1424.4	1158.9	-1.4	151.5	123.2	-103.9	-121.6	1.00	
		0.30	9204.781	-5143.666	-3537.975	-722.0	-455.0	-13.9	146.8	-48.4	-1.5	-164.1	-112.8	1.00	
		0.40	14766.624	-4376.282	-4801.004	-93.9	-1247.9	-1345.4	235.5	-132.7	-143.1	-139.6	-155.7	1.00	
		0.50	16647.156	-4913.546	-4807.852	-313.2	-1783.5	-1747.8	265.5	-189.6	-185.8	-156.7	-153.3	1.00	
		0.60	14948.543	-4905.645	-3965.657	-574.7	-2226.3	-1053.4	238.4	-236.7	-112.0	-156.5	-126.5	1.00	
		0.70	16348.838	-4625.082	-3866.505	-372.7	-2355.4	-2453.0	260.7	-250.4	-260.8	-147.5	-123.3	1.00	
0.80	-7672.520	-7702.597	-3074.230	1053.1	-3050.2	-3018.9	-122.4	-324.3	-321.0	-245.7	-98.1	1.00			
0119-0183	1	0.00	5904.498	-2654.567	-3239.130	9.7	1124.7	1028.4	94.2	119.6	109.3	-84.7	-103.3	0.58	

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***** COLLAPSE SOLUTION SUMMARY *****

LOAD SEQUENCE 1

INCR	LOAD CASE	LOAD FACTOR	NO. LOOPS	* MAXIMUM DEFLECTION * CM	* MAXIMUM ROTATION * JOINT DOF	** MAXIMUM ROTATION ** ROT. JOINT DOF	** SOLUTION DATA ** MAX. JOINT DOF DIGITS	*** REACTION SUMMATION *** FX KN	FY KN	FZ KN		
1	SW	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
2	2	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
3	3	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
4	4	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
5	5	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
6	6	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
7	7	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
8	8	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
9	9	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
10	10	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
11	11	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
12	12	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
13	13	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
14	14	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
15	15	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
16	16	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
17	17	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
18	18	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
19	19	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
20	20	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
21	21	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
22	22	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
23	23	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
24	24	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
25	25	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
26	26	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
27	27	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
28	28	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
29	29	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
30	30	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
31	31	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
32	32	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51
33	33	1.00	1	-0.302	0049 DZ	-0.0004049	0117 RV	1	0135 DZ	0.00	0.00	729.51

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226	226	1.00	0009-0147	0.00	-6.46	119.93	-225.45	83.85	-40.06	1.00				
226	226	1.00	0100-0162	0.00	-8.73	85.40	-259.17	72.62	-8.12	1.00				
226	226	1.00	0101-0177	0.00	-2.12	-340.52	-52.35	-35.10	17.12	1.00				
226	226	1.00	0102-0148	0.00	-22.58	83.28	-242.16	85.78	-25.02	1.00				
226	226	1.00	0103-0163	0.00	-51.34	133.83	-235.87	47.04	-3.11	1.00				
226	226	1.00	0104-0178	0.00	-79.51	-271.24	40.64	-24.73	4.97	1.00				
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ID=00000000														
***** JOINT FAILURE SUMMARY REPORT *****														
LOAD SEQUENCE 1														
INCR	JOINT	CHORD	BRACE	PERCENT	PERCENT	PERCENT	*** APPLIED STRESSES ***			** ALLOWABLE STRESSES **			UNITY	
FAILED	JNT	JNT	T&Y	X	K		AXIAL	OUT-PLN	INPLANE	AXIAL	OUT-PLN	INPLANE	CHECK	
							N/MM2	N/MM2	N/MM2	N/MM2	N/MM2	N/MM2		
202	0157	0156	0142	0.00	100.00	0.00	36.66	-193.25	-90.72	89.22	224.89	318.73	1.106	
204	0157	0156	0085	100.00	0.00	0.00	-57.55	-39.57	166.45	43.28	142.66	15.90	11.804	
205	0156	0155	0082	100.00	0.00	0.00	-19.21	1.40	305.15	57.55	164.01	97.76	3.256	
206	0158	0157	0088	100.00	0.00	0.00	-32.27	-12.01	99.38	52.03	155.75	66.10	1.944	
208	0040	0039	0068	100.00	0.00	0.00	22.65	62.42	301.38	56.67	296.32	56.58	5.521	
209	0043	0042	0104	100.00	0.00	0.00	-35.25	35.34	-236.66	69.78	297.88	60.89	4.187	
210	0142	0141	0084	100.00	0.00	0.00	-21.77	45.71	301.69	48.31	150.19	44.75	6.988	
211	0154	0153	0076	100.00	0.00	0.00	72.01	-0.55	160.28	35.29	142.22	14.19	13.338	
212	0108	0107	0168	100.00	0.00	0.00	-41.61	-29.00	310.33	64.70	169.09	65.45	5.213	
213	0123	0122	0017	100.00	0.00	0.00	0.49	-26.64	304.83	33.81	139.52	3.86	78.959	
214	0153	0152	0073	100.00	0.00	0.00	49.18	-27.00	-168.78	37.76	146.72	31.47	6.668	
215	0106	0105	0014	100.00	0.00	0.00	31.35	245.24	210.72	33.77	139.45	3.59	59.691	
216	0153	0152	0168	100.00	0.00	0.00	13.82	1.24	314.17	52.69	168.43	62.47	5.113	
217	0173	0172	0089	100.00	0.00	0.00	-42.28	-0.69	295.29	43.81	143.45	18.93	16.546	
218	0110	0109	0030	100.00	0.00	0.00	0.15	19.87	304.04	33.35	138.68	0.61	495.095	
219	0174	0173	0092	100.00	0.00	0.00	-29.16	7.25	305.00	45.64	146.18	29.39	10.866	
220	0124	0123	0018	100.00	0.00	0.00	8.63	-46.16	301.00	33.91	139.71	4.57	65.969	
221	0166	0165	0069	100.00	0.00	0.00	64.48	-520.00	-465.03	-997.82	35.28	-4648.75	14.745	
222	0112	0111	0032	100.00	0.00	0.00	-5.16	94.20	291.01	40.52	138.53	0.04	6797.247	
223	0027	0014	0107	100.00	0.00	0.00	-224.99	388.85	-1866.03	76.47	221.35	2.61	719.082	
224	0160	0159	0094	100.00	0.00	0.00	0.67	-13.04	313.19	37.00	145.34	26.17	11.970	
225	0116	0115	0036	100.00	0.00	0.00	-3.64	-92.03	230.89	41.89	140.58	7.92	29.185	